

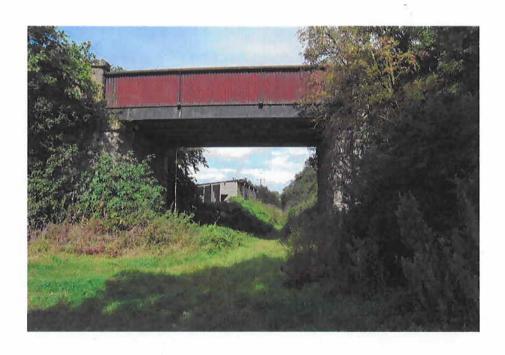
BRB (Residuary) Ltd Major Works Programme 2009/2012

VAR9/2602 ASSESSMENT PROGRAMME

BD21 ASSESSMENT AND INSPECTION REPORT

Red Barn Overbridge, Rushbury, Shropshire

BRIDGE REF: BUI/26



January 2010



Document control sheet

BPP 04 F8

B12360AH - BUI/26

Client:

BRB (Residuary) Ltd

Project:

Major Works Programme 2009/2012

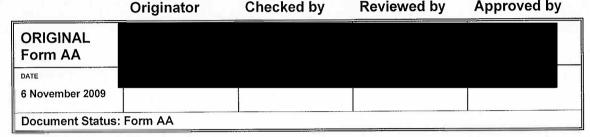
Document Title:

VAR9/2602 Assessment Programme **BD21 Assessment and Inspection**

BUI/26

Approved by Reviewed by Checked by

Job No:



REVISION	NAME	NAME	NAME	NAME
Form BA				
DATE				
27 January 2010				
Document Status	: Form BA	0		

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DATE	SIGNATURE	SIGNATURE	SIGNATURE	SIGNATURE
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1 General Description and Structural Details

1.1 Introduction

Jacobs was appointed by BRB(R) to conduct the site survey at BUI/26 in sufficient detail to provide data for BD21 assessment work.

Structural Soils Ltd excavated two trial pits on the centre line of the road, one at the mid-span and the other one over the north abutment to expose the top flange of the centre girder.

1.2 Location and General Description

Bridge BUI/26 carries a public bridleway, also used for field access, over the track bed of the former Buildwas to Marsh railway at Rushbury in Shropshire.

The bridleway is unpaved. The bridge deck itself is concrete with no additional surfacing. Overall width between the parapets is about 5.43m (Photo 3 & 4).

The OS grid reference is SO 513 912.

The bridge was constructed in 1867. Pre-cast concrete jack arches appear to have replaced the original brick jack arches circa 1960.

1.3 Construction type

The structure is a square, single span overbridge. The clear span is 7.58m (24' – 10") (photo 1).

The bridge comprises of three internal longitudinally spanning cast iron girders, spaced at 4' - $4\frac{1}{4}$ " (1.327m) centres and two cast iron edge girders (Photo 6). Precast concrete arches span between the bottom flanges of the girders to form the bridge deck.

The internal girders have an overall depth of 21" (533mm) at mid span and 16" (406mm) at supports. The edge girders have an overall depth of 20" (508mm) throughout.

Three tie bars fix the edge girders to the internal girders at mid-span and quarterspan.

The abutments and wingwalls are constructed from random size stones irregularly coursed.

The parapets are corrugated steel sheeting with cast apexes on top. The parapets are supported by masonry pilasters at the ends and two stiffeners along the span. The stiffeners are connected to the top flange of the edge girders and the outer face of the corrugated steel parapet.



2 Existing Information Search

2.1 Services Search

Documentation obtained by Structural Soils Ltd is included in Appendix B.

2.2 SI Results

Trial pits were excavated as part of the survey and were located at the mid-span and the north abutment over the centre girder.

Data on the trial pit and a description of the investigation is included in Appendix C.

2.3 Existing Drawings

There is one bridge drawing available showing deck section, elevation and plan of the bridge. The drawing shows the bridge in its original form with brick jack arches. The bridge deck was reconstructed with pre-cast concrete arches and the girders may have been re-bedded at the same time accounting for minor dimensional differences.



3 Structure Condition

3.1 General

The survey and inspection for BD21 assessment were undertaken on Monday 24 August 2009. There were heavy showers with sunny intervals and temperature was about 20°C.

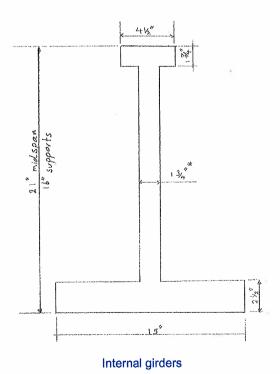
It was possible to park on the road at the bridge. There is no regular traffic use. Access to the formation was gained through the field access track at the north west side of the bridge.

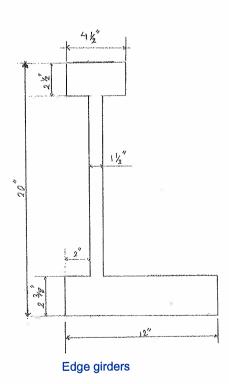
3.2 Main superstructure

3.2.1 Longitudinal girders

The edge girders are generally in satisfactory condition with no significant section loss; though they have some calcareous staining to the inside leg of the bottom flanges (Photos 5&6).

The internal girders are in similar condition. They show no section loss throughout the bottom flanges, with only some calcareous staining and small stalactites (Photo 6).





3-1



3.2.2 Precast concrete arches

The precast concrete arches area in good condition with no major defects (Photo 6).

3.2.3 Tie bars

The tie bars in the west bay are in satisfactory condition with minor surface corrosion (Photo 6).

The tie bars in the centre bays have significant corrosion and section loss. The residual diameter of their corroded section is about 3/4"

The tie bars in the east bay have severe section loss due to corrosion. The first tie bar from the north is corroded and broken. The residual diameter of the other two tie bars is about $\frac{1}{2}$ (Photo 7).

3.3 Abutments

The abutments are constructed from random size stones irregularly coursed.

The south abutment is in satisfactory condition. There is a crack starting from the east bedding stone stretching diagonally towards the quoin stone at ground level. There is a sapling growing about 1m down from the bedding stone at the east corner. Mortar filling is generally good, except at the east quoin stones where there is a significant mortar loss (photo 9).

The north abutment is also in satisfactory condition. Mortar filling is generally in good condition, except the east corner where there is some moderate mortar loss. One of the bedding stones under the first internal girder from the east appears to be spalled (photo 8).

3.4 Wingwalls

The wingwalls are constructed from random size stones irregularly coursed.

The north east wingwall is obscured by vegetation. From what can be seen, it appears to be in satisfactory condition (photo 11).

The north west wingwall is generally in satisfactory condition. There is some mortar loss in the abutment corner.

The south east wingwall has some vegetation growing on the corner next to the abutment and also on the pilaster. There is some mortar loss in the lower courses (Photo 12).

The south west wing wall is in good condition. There is some mortar loss in the top corner next to the abutment (photo 13).

3.5 Parapets

The parapets have surface corrosion throughout, but there is no significant section loss.



3.6 Formation

The formation is reverting to natural vegetation. There is an infrequently used unofficial footpath going under the bridge to the east. (Photo 1).

3.7 Road surface

The bridge is on an unmade track used for access to the fields (Photos 3 & 4). The bridge deck itself is concrete which is showing some signs of surface deterioration and shallow rooted vegetation has taken hold. Road surface condition is rated as "poor" for BD21 bridge specific live loading.



4 Assessment to BD21

4.1 Structural Parts checked to BD21

The following parts of the bridge were checked to BD21:

- Internal girders: Bending in tension, bending in compression, shear
- Edge girders: Bending in tension, bending in compression, shear
- Jack arches

4.2 Methodology

BD21 vehicle loading was applied to the internal girders using the simple distribution methods outlined in Chapter 2 of BA 16/97. As the concrete surface occupies the entire width of the bridge, the carriageway loading was also applied to the edge girders using the same method. No checks for accidental vehicle loading were deemed necessary as carriageway loading applies on all girders.

The embedment of the internal cast iron girders in concrete permits an increase in the section modulus for live load by D/d in accordance with BD21/01 Clauses 7.12 to 7.15.

The assessment was based on the current condition of the structure as determined by the inspection. Specific allowance for recorded section losses was made to the appropriate component of the relevant elements.

Determination of the adequacy of the jack arches was based upon the empirical method described in Bridgeguard 3 Current Information Sheet No 22 (Pro-forma for the empirical assessment of brick, masonry and concrete jack arches and associated ties), though the pre-cast concrete jack arches need special consideration.

The substructure was assessed qualitatively.

4.3 Results

Element: Internal girders

Span = 7.71m

26 tonne loading K factors for various road surface and HGV flow combinations:

Road Surface	High (H)	Med. (M)	Low (L)
Good (g)	0.79	0.77	0.75
Poor (p)	0.89	0.87	0.85

18 tonne loading K factors for various road surface and HGV flow combinations:

		HGV Flow		
Road Surface	High (H)	Med. (M)	Low (L)	
Good (g)	0.64	0.62	0.59	
Poor (p)	0.73	0.71	0.64	



C = Available live load capacity / Live load capacity required for Adjusted HA loading and relates directly to the K factors in Figures 5.2 to 5.7 of BD21/01.

C > K = 0.64 for 18 tonne loading (Lg) Low HGV flow, poor road condition.

Action	Location	Dead load effect	HA adjusted LL effect	Assessed live load resistance	C factor	Live load rating
Bending (tension)	Mid-span	105.9 kN.m	291.2 kN.m	200.1 kN.m	0.687	18 tonnes
Shear	Support	54.9 kN	151.8 kN.m	420.9 kN	2.77	40 tonnes

Element: Edge girders

		HGV Flow		
Road Surface	High (H)	Med. (M)	Low (L)	
Good (g)	0.64	0.62	0.59	
Poor (p)	0.73	0.71	0.64	

7.5 tonne loading K factors for various road surface and HGV flow combinations:

		HGV Flow	
Road Surface	High (H)	Med. (M)	Low (L)
Good (g)	0.35	0.34	0.33
Poor (p)	0.40	0.38	0.37

Group 1 FE loading K factors for all combinations: **0.49** Group 2 FE loading K factors for all combinations: **0.25**

Action	Location	Dead load effect	HA adjusted LL effect	Assessed live load resistance	C factor	Live load rating
Bending (tension)	Mid-span	69.7 kN.m	247.9 kN.m	137.7 kN.m	0.555	7.5 tonnes Grp. 1FE
Shear	Support	36.0 kN	128.0 kN	448.6 kN	3.5	40 tonnes

C > K = 0.37 for 7.5 tonne loading (Lg) Low HGV flow, poor road condition.

C > K = 0.49 for Group 1 fire engine loading.

Element: Jack arches and tie rods

The pre-cast concrete jack arch units are considered to be adequate for full local wheel effects although they do not conveniently fit with the criteria required in the Bridgeguard CIS No.22 empirical method of assessment. The combination of the arch unit and infill concrete should give adequate resistance.

The tie bars in the west bay are satisfactory. They are just about compliant for full 40 tonne loading but quite acceptable for the restricted capacity of the principal girders. Those in the east bay are badly corroded and one is fractured and need to be replaced. They are non-compliant in their current state though it is noted that vehicles do not normally track on this part of the deck.



Element: Substructure

The abutments show no signs of structural distress. By qualitative assessment, they are rated at 40 tonnes Assessment Live Loading.



5

Conclusions and Recommendations

The carriageway width and hence the number of notional lanes on the bridge is subject to interpretation. The width between the parapets is 5.49m which would indicate two notional lanes, but the width is further restricted between the pilasters giving an effective with less than 5m and hence single lane loading. Given the very limited use of the bridge by vehicles and the encroachment of vegetation leading to a single vehicle track in the centre of the bridge, it appeared reasonable to adopt single lane loading.

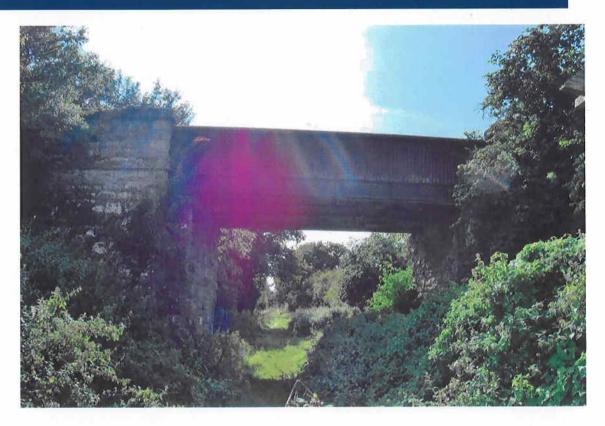
The capacity of the bridge is limited by the edge girders to 7.5 tonnes, but practically 18 tonnes can be achieved in its current pattern of use, as this is the capacity of the three central girders. To formalise the assessment at 18 tonnes it would be necessary to install kerbs or other barriers to limit the carriageway loading position. In this instance accidental vehicle loading would have to be considered on the verges and again would be limited by the capacity of the edge girders.

The tie bars in the east edge bay of the bridge are corroded and the 7.5 tonne limit is dependent on these tie bars being replaced. The tie bars on the west edge of the bridge are adequate for maximum AW vehicle loading.

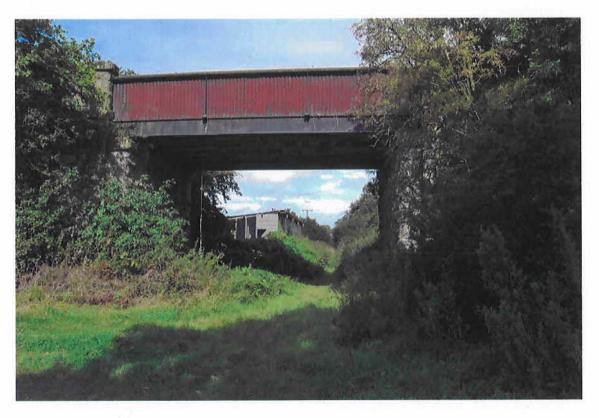
Any repairs or strengthening of the bridge needs to consider the very limited use made of the bridge by vehicles; not more than occasional field access by agricultural equipment.



Appendix A - Photographs



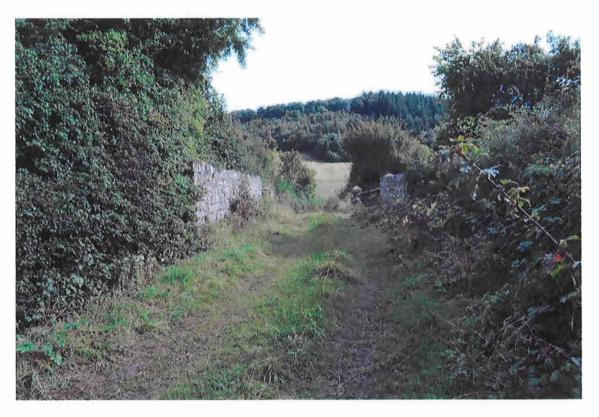
1. East elevation



2. West elevation



3. Road over bridge looking north



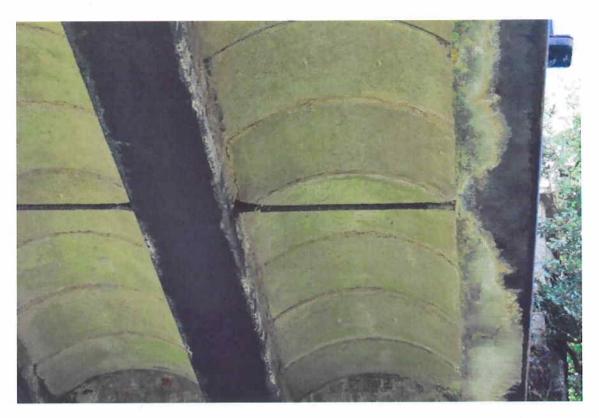
4. Road over bridge looking south



5. West edge girder and parapet



6. Bridge deck soffit



7. Precast concrete arches and broken tie bar in the east bay



8. North abutment



9. South abutment

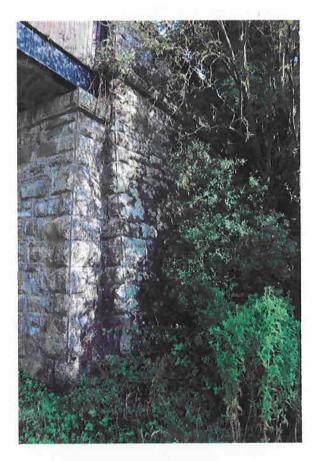


10. North west wingwall

Form BA BUI_26 BD21 report.doc



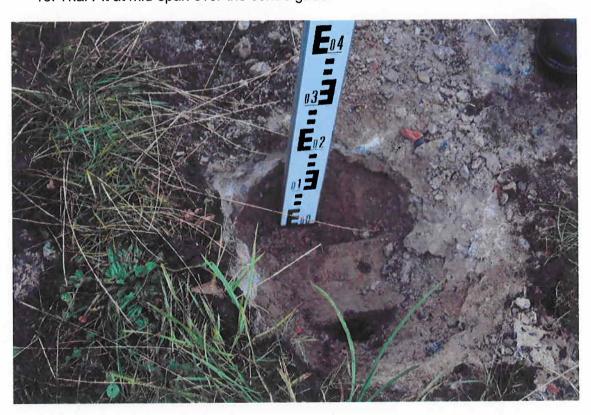
11. South east wingwall



12. South west wingwall
Form BA BUI_26 BD21 report.doc

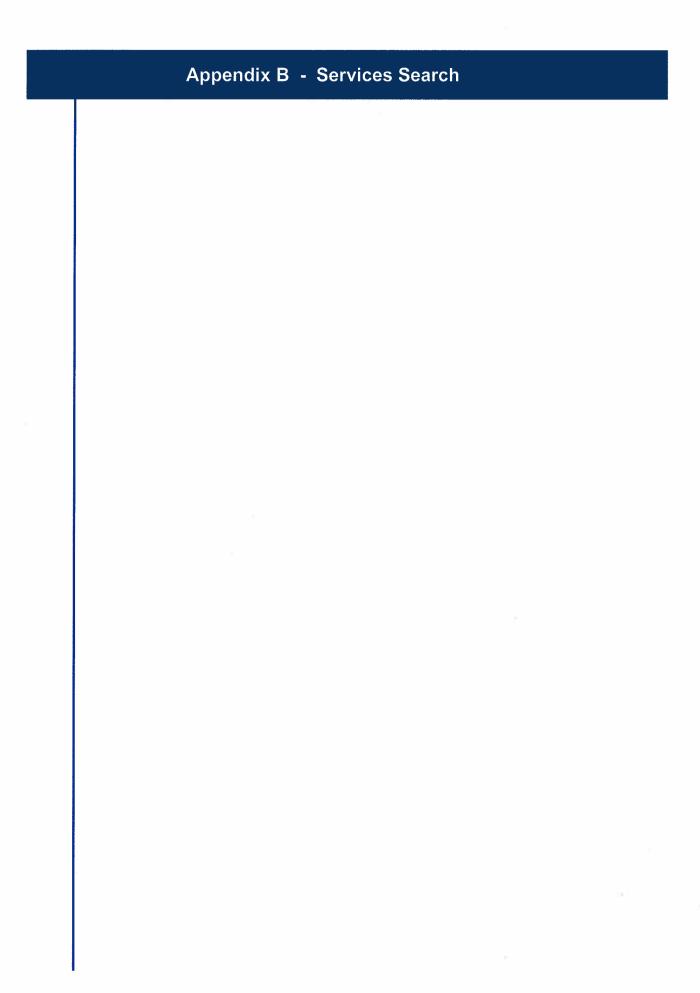


13. Trial Pit at mid-span over the centre girder



14. Trial pit at north abutment over the centre girder





National One Call powered by PlanToDig.com

Response Summary

enquiries@national-one-call.co.uk
Tel: 0844 800 9957 Fax: 0845 280 2040

Enquiry Number	EQ/UVSJD484	Service	Retriever
Location of Enquiry	at Grid Refs 351353 291257 St Rushbury, Shropshire, SY6 7		East of Darby Lane, Sth of
	Status: Af	ffected	
Organisation	Response		
Severn Trent Plc	water		
	Status: Not	Affected	
Organisation	Response		
Affiniti - Kingston Communications	Please find attached details of your recent p affected by the proposed works at the locati date of receipt	plant enquiry. Kingston Colion specified on the notice	mmunications network is not e. This is valid for 3 months from
Dwr Cymru Welsh Water	Not our supply area		
E S Pipelines Ltd	7 July 2009 Reference: EQ/UVSJD484 Dear Rushbury Rd and East of Darby Lane, Sth of has no gas or electricity apparatus in the vice proposed works. ESP are continually laying for 90 days from the date of this letter. If you submit your enquiry. Important Notice Please formerly known as British Gas Connections Yours faithfully,	f Rushbury, Shropshire, I cinity of this site address a new gas and electricity neour proposed works start ase be advised that any enouth, should be sent directly	can confirm that ESP Gas Group Ltd and will not be affected by your etworks and this notification is valid after this period of time, please re- quiries for ESP Connections Ltd,
Energetics	can confirm Energence done have any plant	t located at the address in	question Thanks
Fibrespan Ltd	With regard to your enquiry below, I can coby your proposed works. Kind regards,	onfirm that FibreSpan Ltd.	does NOT have any plant affected
Fujitsu - Orange pcs	Location SOUTH OF RUSHBURY ROAD AND EAST OF DARBY LANE, SOUTH OF RUSHBURY Dated 18-JUL- 09 With reference to your enquiry regarding the above noted location, we are unaware of any ORANGE PCS plant or services supported by Fujitsu in the area indicated in your enquiry. We bring your attention to the fact that whilst we try to ensure the information we provide is accurate, the information is provided Without Prejudice and Fujitsu accepts no liability for claims arising from any inaccuracy, omissions or		
Gamma Telecom	Having examined my records, I can confirm area of your enquiry below:- Regards Ray 0	n that Gamma Telecom ha Gamma Telecom Plant Rec	s no owned apparatus within the cords
Gas Transportation Co	Site Ref: EQ/UVSJD484 Date: 15 July 2009 Re: Sth of Rushbury Rd and East of Darby Lane, Sth of Rushbury, Shropshire. Thank you for your enquiry concerning apparatus in the vicinity of your proposed work. GTC/ENC can confirm that we have no apparatus in the vicinity but please note that other Gas Transporters/Electricity Distributors may have and that you should ensure that all transporters/distributors have been consulted. All future plant enquiries must contain accurate Easting and Northing references to enable us to process your enquiry efficiently. Yours sincerely GTC		
Interoute - Netspher 24 Ltd	we are unaware of any Interoute plant		
Spectrum Interactive plc	In response to your query regarding the reference above, Spectrum Interactive (formerly New World Payphones) our initial thoughts are we have No telephone kiosk onsite at present. Our plant is above ground, and should not affect any underground works, should you come across a telephone kiosk and it's not a BT one then it will be one of ours. If you have any further queries please do not hesitate to contact me on 01442 205538 Melanie Mulvanerty Logistics and Delivery Administrator Spectrum Interactive plc 01442 205534		

THUS plc	We are unaware of any Thus plant in the vicinity of your proposed works. Thanks Streetworks Noticing Officer
Verizon Business	Verizon Business is a licensed Statutory Undertaker. We have reviewed your plans and have determined that Verizon Business (Formally known as MCI WorldCom, MFS) has no apparatus in the areas concerned. If you have any further queries please do not hesitate to call. Yours faithfully Plant Protection Officer
Virgin Media (NTL:Telewest)	virgin media and viatel plant should not be affected by your proposed works
Vtesse Networks Ltd	Your Ref EQ/EEHBY820 EQ/MVQD0283 EQ/MIJEW231 EQ/IMXE0056 EQ/WNVRL555 EQ/MIRHY134 EQ/FCNEB812 EQ/GWFCU088 EQ/NWVFG285 EQ/RPIXI360 EQ/UVSJD484 EQ/EHOKO832 Our Ref 0709/324 0709/325 0709/326 0709/327 0709/328 0709/329 0709/330 0709/331 0709/332 0709/333 0709/334 0709/335 I confirm that Vtesse Networks do not have any plant in the area of your enquiry. Regards Amanda Pearson
	Copyright National One Call 2008 all rights reserved

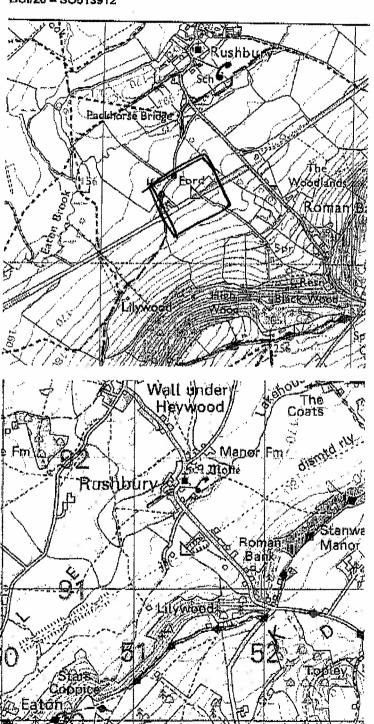
01977552299

SECTION 2 STRUCTURE 2

East of Darby Lane

Sth of Rushbury at 357353, 291257.

BUI/26 - SO513912



EQ/UVSJD484

Search Results



Thank you for your enquiry: LS-090707-VD-545-UME

Subject always to our standard terms and conditions, this enquiry result is valid for 28 days only from the date of enquiry and is based on the confirmed information you entered. If the location of the work changes then a further enquiry must be made. Should the work not be undertaken within 28 days of the enquiry then a further enquiry must be made.

ENQUIRER DETAILS

Name: Mr McMaster Company: PlanToDig

Email: brian.mcmaster@national-one-call.co.uk

ENQUIRY DETAILS

Your reference: EQ/UVSJD484 - Sth of Rushbury

Rd, Shropshire

Your location: 351312 291340

Confirmed location: OS grid reference (351300

291325)

Estimated start date: 20-07-2009

Type of work: Excavations Non Utility - Private

services

Distance covered: 250 metres

NOT IN THE ZONE OF INTEREST

BP Exploration Purbeck Southampton

Pipeline

BPA

Centrica Energy

ConocoPhillips (UK) Ltd

ConocoPhillips Ltd Humber Refinery

Coryton Energy Co Ltd (Gas Pipeline)

E-on UK Plc (Gas Pipelines Only)

Esso Petroleum Company Limited

Geo Networks Limited

Government Pipelines & Storage System

Ineos

Mainline Pipelines Limited Manchester Jetline Limited

Marchwood Power Ltd (Gas Pipeline)

NPower CHP Pipelines

National Grid (National Gas and Electricity

Transmission Networks)

Premier Transmission Ltd (SNIP)

Sabic UK Petrochemicals

Scottish Power Generation Ltd

Shell UK Ltd

Total

Wingas Storage UK Ltd

Thank you for your enquiry, there is no further action necessary.

Please note that the Linesearch.org system only contains information on National Grid - (National Gas and Electricity Transmission Networks) This does not include National Grid's or others local high pressure (above 7 bar) gas pipelines and lower pressure gas pipelines. Details of who to contact for National Grid's distribution assets, including their local high pressure pipelines, can be found via http://www.nationalgrid.com/uk/LandandDevelopment/DDC/GasElectricNW

For other energy network operators' contact details see http://2008.energynetworks.org/

Please quote the Linesearch enquiry reference number in *all* correspondence

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NON AFFECTED PLANT ENQUIRY

KCOM Group Pic Network Engineering Centre Vulcan Street Hull **HU6 7PS**

Tel: 01482 603479

Kingston Communications network is not affected by the proposed works at the location specified below. This is valid for 3 months from date of receipt. For further info please call 01482 603479.

Please send all future enquiries to the postal address above or alternatively they can be e-mailed to highwaysadmin@kcom.com

Kingston Notice Number -

90857

Kingston Planner: JS

Undertaker Name - National One Call Your Reference No: EQ/UVSJD484

Location: SOUTH OF RUSHBURY ROAD AND EAST OF DARBY LANE, SOUTH OF

RUSHBURY, SHROPSHIRE



Working with

netsphere 24

Carrier Network Solutions
On behalf of

interoute

The Old Haybarn Rosebery Mews Mentmore Buckinghamshire LU7 OUE

Tel: +44 (0)1525 630017 Fax:+44 (0)1525 630018

e-mail: plantenquiries@netsphere24.co.uk

8 July 2009

Our Ref: PC/39905/PE2877 Your Ref: EQ/UVSJD484

Dear Sir/Madam,

RE: Interoute Communications Ltd Preliminary Enquiry Scheme (C2) at Grid Refs 351353 291254 South of Rushbury Rd and East of Darby Lane, South of Rushbury, Shropshire SY6 7 (351312 291340

With reference to your enquiry regarding the above site, please note that we are unaware of any Interoute Communications Ltd plant. We would like to bring to your attention that whilst we try to ensure the information we provide is accurate, the information is provided without prejudice and both Netsphere24 Limited and Plancast Limited accept no liability for claims arising from any inaccuracy, omissions or errors contained herein.

Should you require any further information, please do not hesitate to contact us.

Yours faithfully

Plant Enquines Coordinator Planning & Design





Base stations displayed in this square

Single operator GSM technology 0

Single operator UMTS technology 0

Single operator TETRA technology 0

Shared base stations with more than one operator or more than one technology

Data Protection Act 1998 | Freedom of Information Act 2000 | Ofcom website disclaimer | Accessibility at Ofcom | Sitefinder query form

Taylor Roger

From:

retriever@national-one-call.co.uk

Sent:

07 July 2009 12:02

To:

Taylor Roger

Subject:

Dwr Cymru Welsh Water - EQ/UVSJD484

National One Call Enquiry EQ/UVSJD484

Documents / Responses Requested from

Dwr Cymru Welsh Water

Requests are made for the following documents.				
Click the document to see the Plan	Use this section only for marking up to fax /Email			
Document	Affected	Action Taken / Required / Comments		
Water / Drainage Plan	Yes / No			

Click here to see the marked-up plan

If you prefer, you can print this form, mark it up and fax it back to us on 0845 280 2040
If you are faxing this form to us and are not affected, simply highlight 'No'. If you are affected, tell us what action you have taken, or that we should take to obtain the document and if there is a charge made for you providing the document.

Required Date	20/07/2009	Response Deadline 19/07/2009	
-	7.1.1.1.1.1	Response Deaume 17/0//2007	
Enquirer	Structural Soils Ltd		
Contact	Retriever from National One Call		
Email address	retriever@national-one-call.co.uk Click this address to respond by email.		
Postal Address	National One Call, 1 Mill Place, Mill Road Industrial Estate, Linlithgow Bridge, West Lothian, EH49 7TL		
Phone	0844 800 9957	Fax 0845 280 2040	
Work Intention	Works Intended		
Created Date	07/07/2009		
Notice given	13 Days (9 Workdays)		
Location Address	at Grid Refs 351353 29 Sth of Rushbury, Shrop	1257 Sth of Rushbury Rd and East of Darby Lane, shire, SY6 7	
Site Description	site at Grid Refs 351353,291257 - Sth of Rushbury and East of Darby Lane		
Comments	Structure BUI/26 over dismantled railway		
Approximate OS Centre Point	SO 51312 91340 : Easting 351312 , Northing 291340		
Bounding Box	351502,291173 to 351121,291507		
Clic	k here to see	the marked-up plan	

THIS SITE IS OUT OF OUR AREA COUTACT SEVERD TRENT WATER

-7 Mr 5003



CIVILS AND CABLING INSTALLATION SPECIALIST

Instalcom House, Manor Way, Herts, WD6 1QH

Telephone: 0208 731 4600 Fax: 0208 731 4601 Email: plantenquiries@instalcom.co.uk

16th July, 2009

Dear Sir/Madam

Your Ref EQ/UVSJD484. Sth of Rushbury Rd & East of Darby Lane, Shropshire

Our Ref: E7/09 – 0675

With reference to your enquiry regarding the above noted locations, I can confirm that GLOGAL CROSSING (UK) LTD, GLOBAL CROSSING PEC and FIBRENET UK LTD networks **DO NOT** have any apparatus within the immediate proximity of your proposed works.

Instalcom responds to plant enquiries for GLOGAL CROSSING (UK) LTD, GLOBAL CROSSING PEC and FIBRENET UK LTD simultaneously and therefore you only need send one copy of a plant enquiry to cover all of these companies. As we are moving towards a fully electronic database we urge our customers to request plant enquiries by email which will result in a higher level of service, please forward future plant enquiries to plantenquiries@instalcom.co.uk

If you require any further information, please do not hesitate to contact me.

Plant Protection Administrator.

Instalcom Limited Instalcom House, Manor Way, Borehamwood, WD6 1QH

E mail:- plantenquiries@instalcom.co.uk

Phone:- 020 8731 4600 Fax:- 020 8731 4601

www.instalcom.co.uk

National One Call

1 Mill Place
Mill Road Industrial Estate
Linlithgow Bridge
West Lothian

EH49 7TL

Drawing Ref:

Plant Enquiry

Your Letter Date
Your Ref:

Date:

07/07/2009 EQ/UVSJD484 21/07/2009

Dear Sir / Madam.

Enquiry Location:

South of Rushbury Road and East of Darby Lane

Shropshire

13557 21/07/2009

VM/PLE/013557

SY6

Thank you for your enquiry regarding work at the above location.

Virgin Media and Viatel plant should not be affected by your proposed work and no strategic additions to our existing network are envisaged in the immediate future.

Virgin Media

Scimitar Park Courtauld Road

Basildon

Essex SS13 1ND

Courtauld Road

National Plant Enquiries Team

Tel: 0870 888 3116 Opt 2 Fax: 01268 468557

Should your request be in relation to a New Development and you require an estimate to be prepared for Virgin Media to service your proposed development, please submit this request for costs along with site drawings (scale 1:500) to:

New Developments Virgin Media 1 Dove Wynd Strathclyde Business Park Bellshill ML4 3AL

This information is only valid on the date of issue. If your start date is 3 months or more from the date of

this letter, please re-apply for updated information.

Yours faithfully,

National Plant Enquiries Team email: plant.enquiries.team@virginmedia.co.uk

01462817523

ATKINS

PAGE 04/05 Page 1 of 1

Gordon, Dominique

From:

NOC Enquiry

retriever@national-one-call.co.uk

Sent:

07 July 2009 12:02

To:

National Plant Enquiry's

Subject:

Follow Up Flag: Follow up

Cable & Wireless - EQ/UVSJD484

Flag Status:

National One Call Enquiry EQ/UVSJD484

Documents / Responses Requested from

Cable & Wireless

capic a micross					
Requests are made for the following documents.					
Click the document to see the Plan	Use this section only for marking up to fax /Emall				
Document	Affected	Action Taken / Required / Comments			
Telecoms, Plan	Yes / No				

Click here to see the marked-up plan

If you prefer, you can print this form, mark it up and fax it back to us on 0845 280 2040

If you are faxing this form to us and are not affected, starply significant into Ricyon are 991 affected, tell us what action you have taken, or that we should take to entain the document and if there is a charge made for you providing the document. CABLE & WIRELESS PLANT IS

NOT AFFECTED

Required Date	20/07/2009	Respense D454166 2007 500 01454 66 3330	
Enquirer	Structural Soils Ltd		
Contact	Retriever from National One Call		
Email address	retriever@national-one-call.co.uk Click this address to respond by email.		
Postal Address	National One Call, 1 Mill Place, Mill Road Industrial Estate, Linlithgow Bridge, West Lothian, EH49 7TL NEW ROADS & STREET WORKS ACT		
Phone	0844 800 9957	Fax 0845 280 2040 PLANT ENOURY	
Work Intention	Works Intended CABLE & WIRELESS PLANT IS 07/07/2009		
Created Date			
Notice given	13 Days (9 Workdays)	NUT AFFECTED	
Location Address	at Grid Refs 351353 291257 Sth of Rushbut 45Rம் வெக்கோண்ட் 01454 66 33 Darby Lane, Sth of Rushbury, Shropshire, SY6 7		
Site Description	site at Grid Refs 351353,291257 - Sth of Rushbury and East of Darby Lane		
Comments	Structure BUI/26 over dismantled railway		
Approximate OS Centre Point	SO 51312 91340: Easting 351312, Northing 291340		
Bounding Box	351502,291173 to 351121,291507		

Click here to see the marked-up plan

This message has been checked for all known viruses by MessageLabs.

Our Ref: H/03/171540 Your Ref: EQ/UVSJD484

14 July 2009

National One Call 1 Mill Place Mill Road Industrial Estate Linlithgow Bridge West Lothian EH49 7TL



Wales & West House Spooner Close Celtic Springs Coedkernew Newport NP10 8FZ Tŷ Wales & West Spooner Close Celtic Springs Coedcernyw Casnewydd NP10 8FZ

T. 029 2027 8500 F. 0870 1450076 www.wwutilities.co.uk

Dear Sir/Madam,

Re: EXCHANGE OF INFORMATION

Wales and West Utilities acknowledge receipt of your notice on 07/07/2009 advising us of the proposals for

Rushbury Rd and East of Darby Lane

According to our mains records Wales and West Utilities has no apparatus in the area of your enquiry. However Gas pipes owned by other GT's and also privately owned may be present in this area. Information with regard to such pipes should be obtained from the owners.

Safe digging practices, in accordance with HS(G)47, must be used to verify and establish the actual position of mains, pipes, services and other apparatus on site before any mechanical plant is used. It is your responsibility to ensure that this information is provided to all persons (either direct labour or contractors) working for you on or near gas apparatus.

If you have any further enquires please contact me on the number below.

Yours faithfully,

Admin Assistant

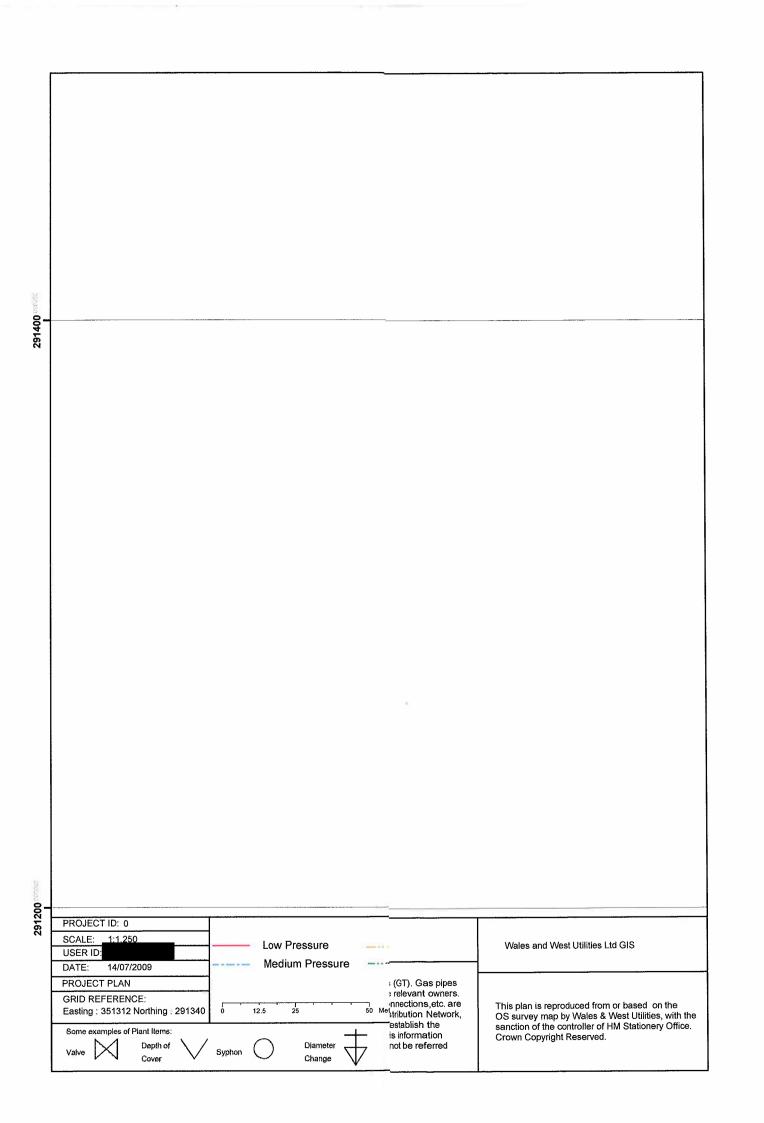
Admin. Assistant Network Records 02920 278845

> 24 hour gas escape number Rhif 24 awr os bydd nwy yn gollwng

0800 111 999*

0000 111 333...

NP/v.21-Mar2003



Our Ref:

WM/18.07.09/ag058/77406

Your Ref:

EQ/UVSJD484

Date:

18 July 2009

National One Call
Mill Place
Mill Road Industrial Estate
Linlithgow Bridge
West Lothian
Glasgow, EH49 7TL.

Dear Sir / Madam.

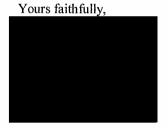
Re: Proposed Works Enquiry South Of Rushbury Rd And East Of Darby Lanerushbury, Shropshire, SY6 National Grid acknowledges receipt of your notice of your intention to carry out work at the above location.

IMPORTANT NOTE: This response is for the Gas Distribution Network ONLY. You MUST also obtaininformation local Electricity Distribution Networks and the National Transmission Gas and Electricity Networks. Please refer to the enclosed "Work Safely in the Vicinity..." leaflet or on www.nationalgrid.com

According to our records National Grid has no gas mains in the area of your enquiry.

Gas pipes owned by other Gas Transporters and also privately owned may be present in this area. Information with regard to such pipes should be obtained from the owners.

If you have any further enquires please contact the Telephone number below.





Our Ref: /NNHC/2006

Your Ref:

Date: As

As Post Mark

National Notice Handling Centre PP 3WW18, Telecom House, Trinity Street, Hanley, Stoke-on-Trent, ST1 5ND.

Freephone: 0800 800865

Freephone 0800 800 865

Dear Customer,

NR & SW ACT 1991 - PROPOSED WORKS AT:

Prior to commencement of work: for free onsite guidance and accurate up to date location of BT plant please contact our Plant Protection Service by the following methods

Tele 0800 9173993

Fax 01332 578650

Email Dial before you dig DBYD@openreach.co.uk Visit the website www.dialbeforeyoudig.com

Thank you for your letter of

describing the above proposals.

Enclosed are copies of our drawing marked up to show the approximate locations of BT apparatus which is present in the immediate vicinity of your works. It is intended for general guidance only. No guarantee is given of its accuracy.

It should not be relied upon in the event of excavations or other works made near to British Telecommunications plc apparatus which may exist at various depths and may deviate from the marked route.

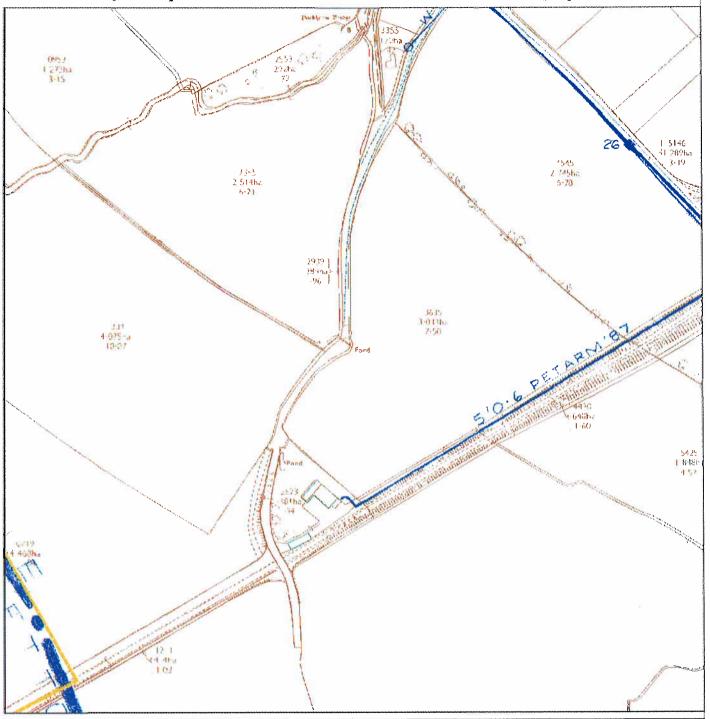
To avoid damage it is recommended that mechanical excavators or borers are not used within 600mm of British Telecommunications plc plant. If scaffolding is erected, please ensure that our equipment is not enclosed, blocked, covered or otherwise obstructed by the scaffolding.

In the event of BT apparatus being in the area of works we recommend that your plant/vehicle crossing is either resited, or apply for a budget estimate by submitting detailed plans to the above address, these will be forwarded to the appropriate department for their comments.

Please ensure you quote our reference on any future correspondence.

Yours faithfully,

Maps by email Plant Information Reply



IMPORTANT WARNING

information regarding the location of 8T appendixs is given for your assistance and is intended for general guidance only.

No guarantee is given of its accuracy.

It should not be relied upon in the event of excavations or other works being made near to BT appendix which may exist at various depths and may deviate from the marked route.

DIAL BEFORE YOU DIG

FOR PROFESSIONAL ON SITE ASSISTANCE PRIOR TO COMMENCEMENT OF EXCAVATION WORKS

ADVANCE NOTICE RECRIRED (Office bours Meaday-Friday 08.00 to 17.00)

Tel 0900 9173993 E-mail dłysi@openressb.co.uk Websila: www.dralbeforeyeudig.com

Reproduced from the Ordnance Survey map by 8T by pemassion of Ordnance Survey on behalf of the Controller of Her Majesty's Stationary Office (C) Crown Copyright British Telecommenications ptc 100026040

Other proposed plant is shown using dashed lines. BT symbols not listed above may be disregarded. Existing BT plant may not be recorded. Information valid at the time of preparation.



BT ref: SUY13010X

Map reference (centre): SO5130091325

Issued: 07/07/09 13:09:57



National One Call

1 Mill Place Mill Road Industrial Estate Linlithgow Bridge West Lothian EH29 7TL

Your Ref: EQ/UVSJD484

Our Ref: W0709/53395

Date: 09/07/2009

Dear Sir/Madam,

Location: South of Rushbury Road

Further to your request dated 07/07/2009 we enclose a copy of our Composite Mains Record(s) showing our plant in the vicinity of the above location.

Please note that cable positions are approximate only, a cable locator and/or careful use of hand tools should be used to verify the exact locations of all cables, including service cables. The cost of rectifying any damage will be charged. The information included on this plan should not be referred to beyond 3 months from the date printed on the plan.

You should ensure that your activities are carried out in accordance with Health and Safety legislation. Should your works be within the vicinity of any Underground/Overhead equipment we would request that it be carried out in accordance with our guidelines as laid down within the attached 'GUIDANCE' document.

Damage to underground cables and contact with overhead lines can cause fatal or severe injury. In the event of any cable damage or contact with an overhead line, please keep people clear and immediately telephone 0800 0152059. This will enable us to take action to control risks to people, limit the effects of damage and consequent cost of repair.

If you require further information relating to any legal rights which the Company may have over this land please contact the Wayleave Department on the telephone number stated on the 'GUIDANCE' document.

Central Networks

Toll End Road Tipton West Midlands United Kingdom DY4 0HH central-networks.co.uk

Bureau Services T 02476 186 502 F 0121 522 6176 AIMBureauServices @central-networks.co.uk

Central Networks East plc No 2366923

Central Networks West plc No 3600574

Central Networks Services Limited No 3600545

Registered in England and Wales

Registered Office: Westwood Way Westwood Business Park Coventry, CV4 8LG



If you have requested additional information a copy of your letter will have been forwarded to the following person/department:

Department:	
Telephone:	

Should you require any further information, please contact AIM Bureau Services.

We note that you have agreed to pay for paper copies of our records and our invoice will be sent to you in due course.

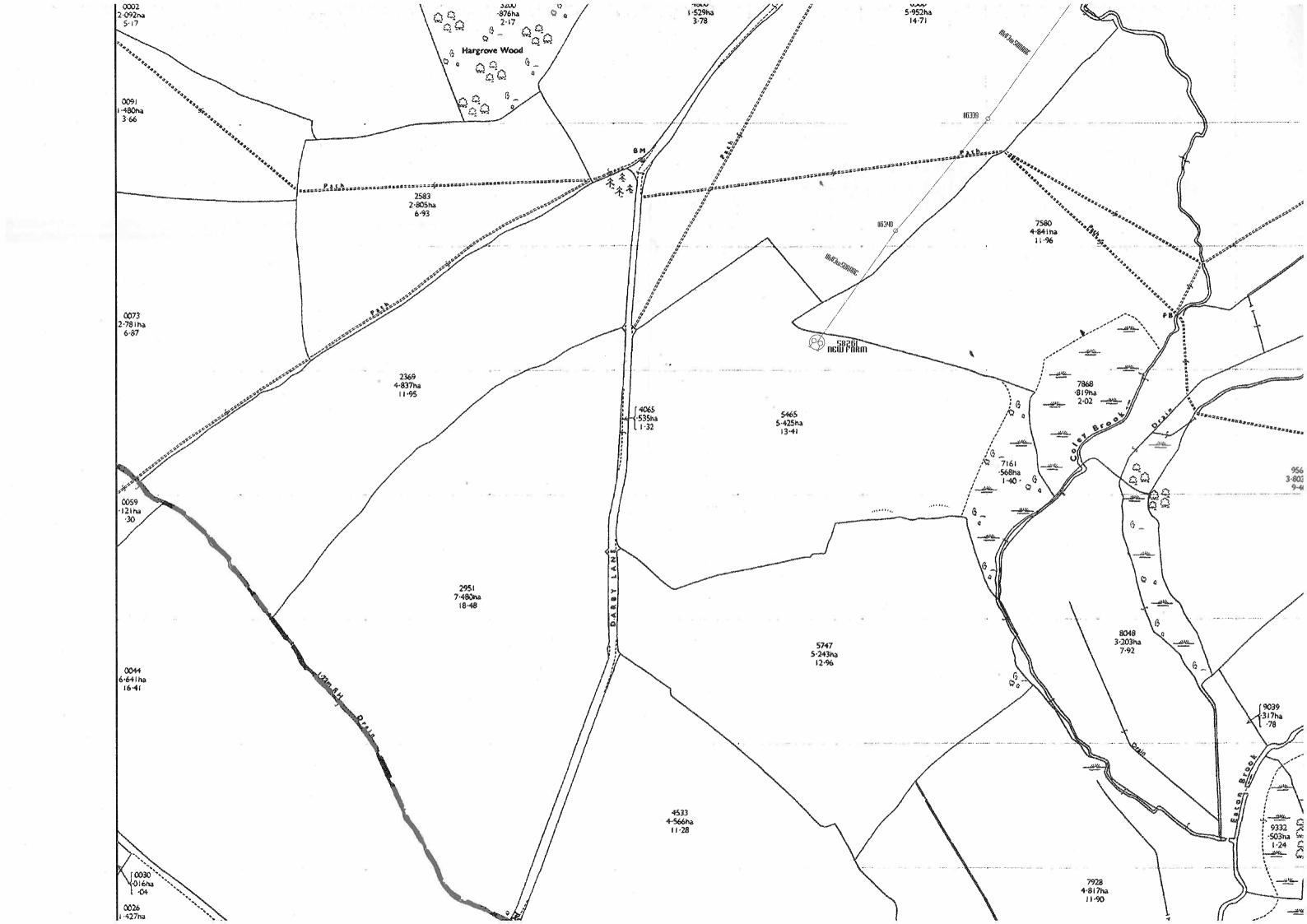
Yours faithfully

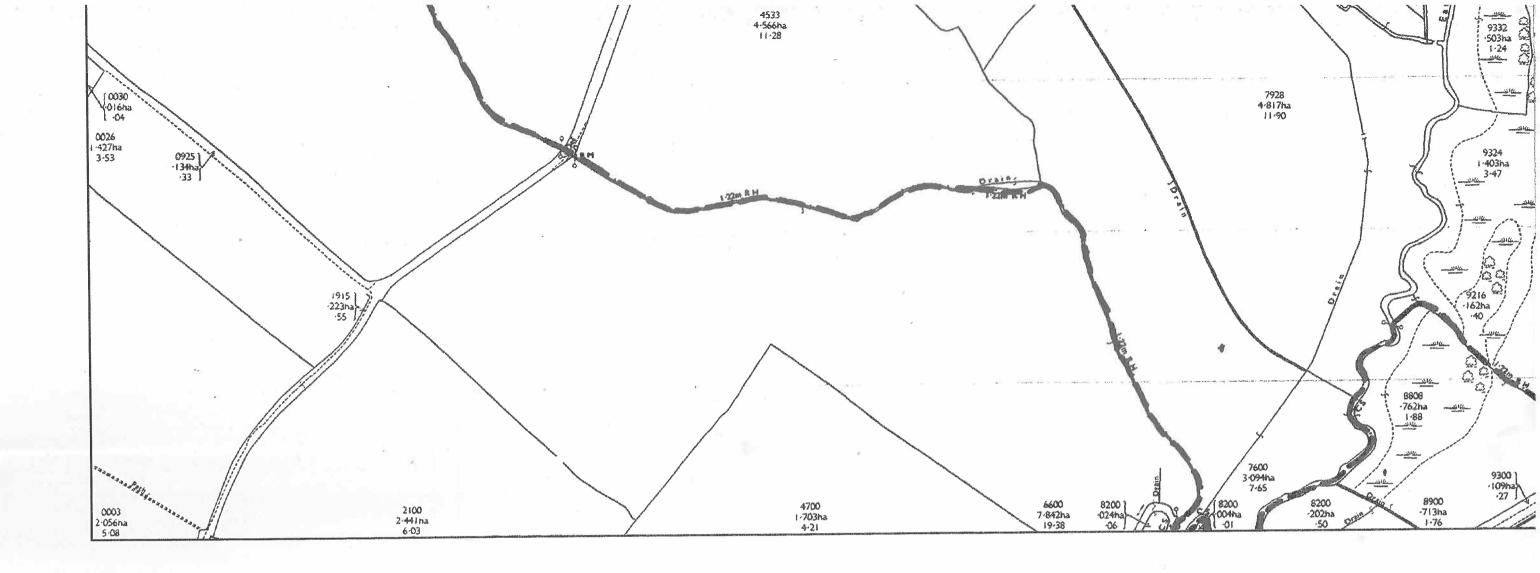
CENTRAL NETWORKS DATA SERVICES

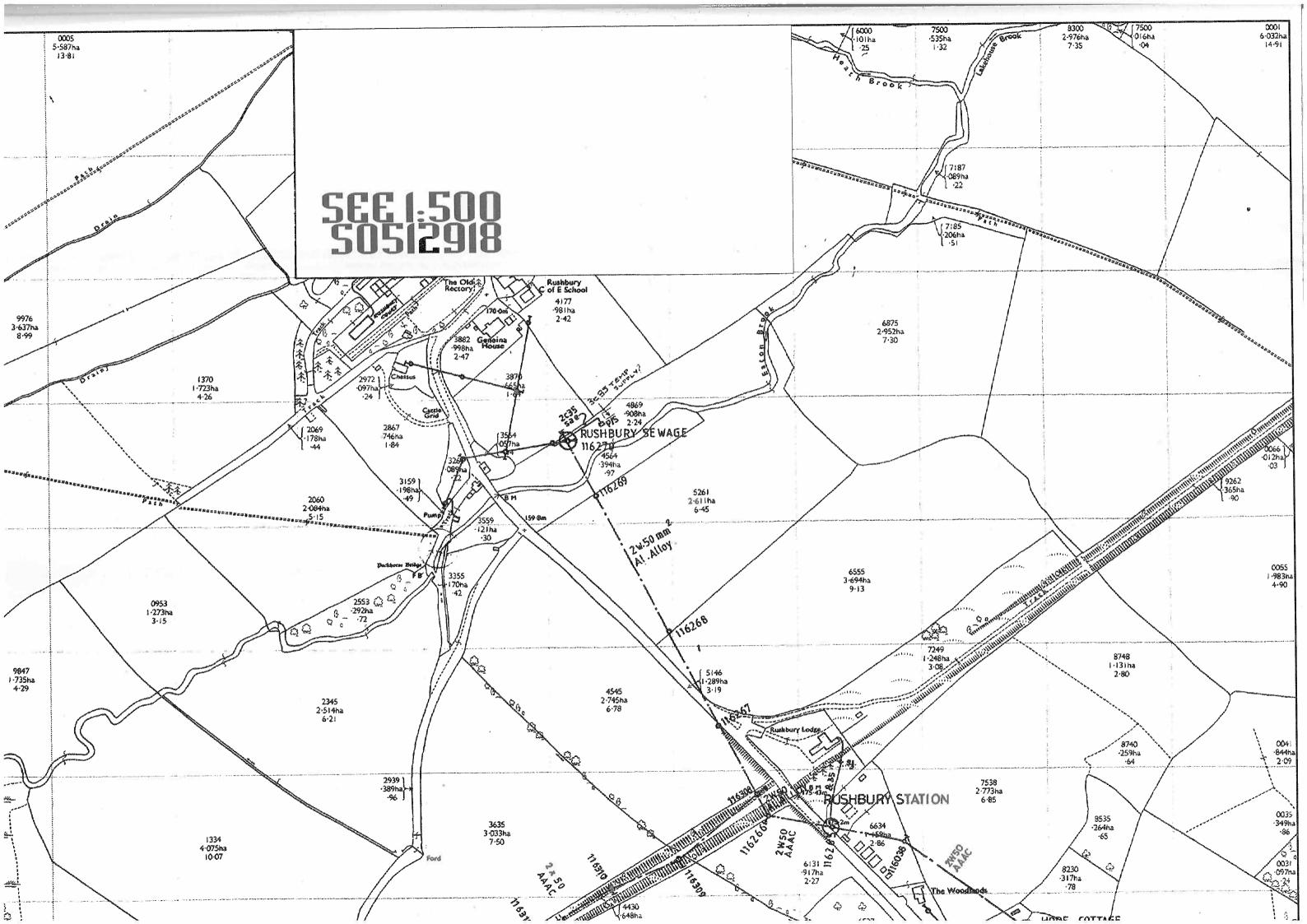
Enclosures:

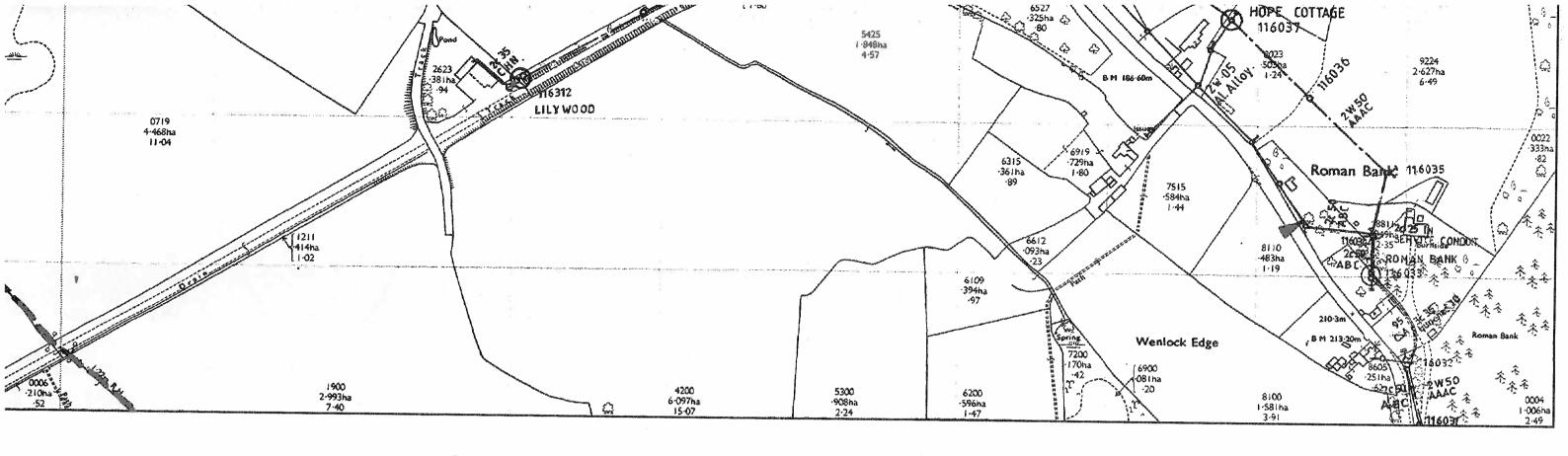
- · Guidance documents
- Requested Drawings

Please refer to enclosed information before carrying out any work









BOUNDARY SEVERN/NORTHERN

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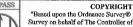
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Central Networks Central Networks Data Services
Bureau Services
West Midlands DV4 0HH
T022/6 186502
F0121 522 6176

DRAWING No.		SCALE	1:2500
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© Crown copyright [Central Networks Licence No. 100017838]"
This drawing must not be copied without the prior permission of Central Networks.

IMPORTANT NOTE

CABLE DEPTHS ARE NORMALLY BETWEEN 0.4 AND 1.5 METRES. THOSE MARKED * HAVE BEEN LAID AFTER 1.10.88 GENERALLY AS FOLLOWS

.,	THE THE PERSON OF THE PERSON O	O GENERO CIAM PED I OL
	LV INCLUDING SERVICES	0.4 - 0.6 METRE
	HV	0.5 - 0.9 METRE
	ALL ROAD CROSSINGS	0.6 - 1.0 METRE
	33-66-132kV	0.9 - 1.5 METRE

ANY DEPTHS SHOWN ON THIS PLAN ARE SPECIFIC TO THAT POINT, AT THE DATE INDICATED OR TIME OF INSTALLATION TO THE BEST OF OUR KNOWLEDGH AND BELIEF.

NOTE THERE MAY BE WIDE VARIATIONS FROM NORMAL OR EVEN INDICATED DEPTHS FOR VARIOUS REASONS E.G. CHANGED SURFACE LEVELS AND UNDERGROUND OBSTRUCTIONS. THEREFORE ALL CABLES IN THE VICINITY OF WORKS MUST BE CAREFULLY LOCATED ON SITE. GUIDANCE FOR SAFE WORKING PROCEDURES IS CONTAINED IN HEALTH AND SAFETY EXECUTIVE GUIDANCE, BOOKLET HISG47 AVAILABLE FROM HISB BOOKS

NO LIABILITY IS ACCEPTED BY CENTRAL NETWORKS FOR ANY ERROR IN OR OMISSION FROM THE PLAN WHISTHER DUE TO NEGLIGENCE OR OTHERWISE THIS SHALL NOT AFFECT LIABILITY FOR DEATH OR PRESONAL INJURY RESULTING FROM NEGLIGENCE.

V4.2 WRG 050607

Severn Trent Water

SEVERN TRENT WATER Ltd

Waterworks Road Edgbaston Birmingham B16 9DD

> Tel Fax

0845 601 6616 0121 452 3569

Direct 0845 601 6616

Line

Contact Plan to Dig Our Ref RMC38578

Apparatus Location Enquiry

Further to your enquiry re: 6 sites All Saints Street B18 Mythe Road GL20 Klondyke Road LE14 Bristol Road GL1 Bushbury Road SY6 and Langton Road LE16

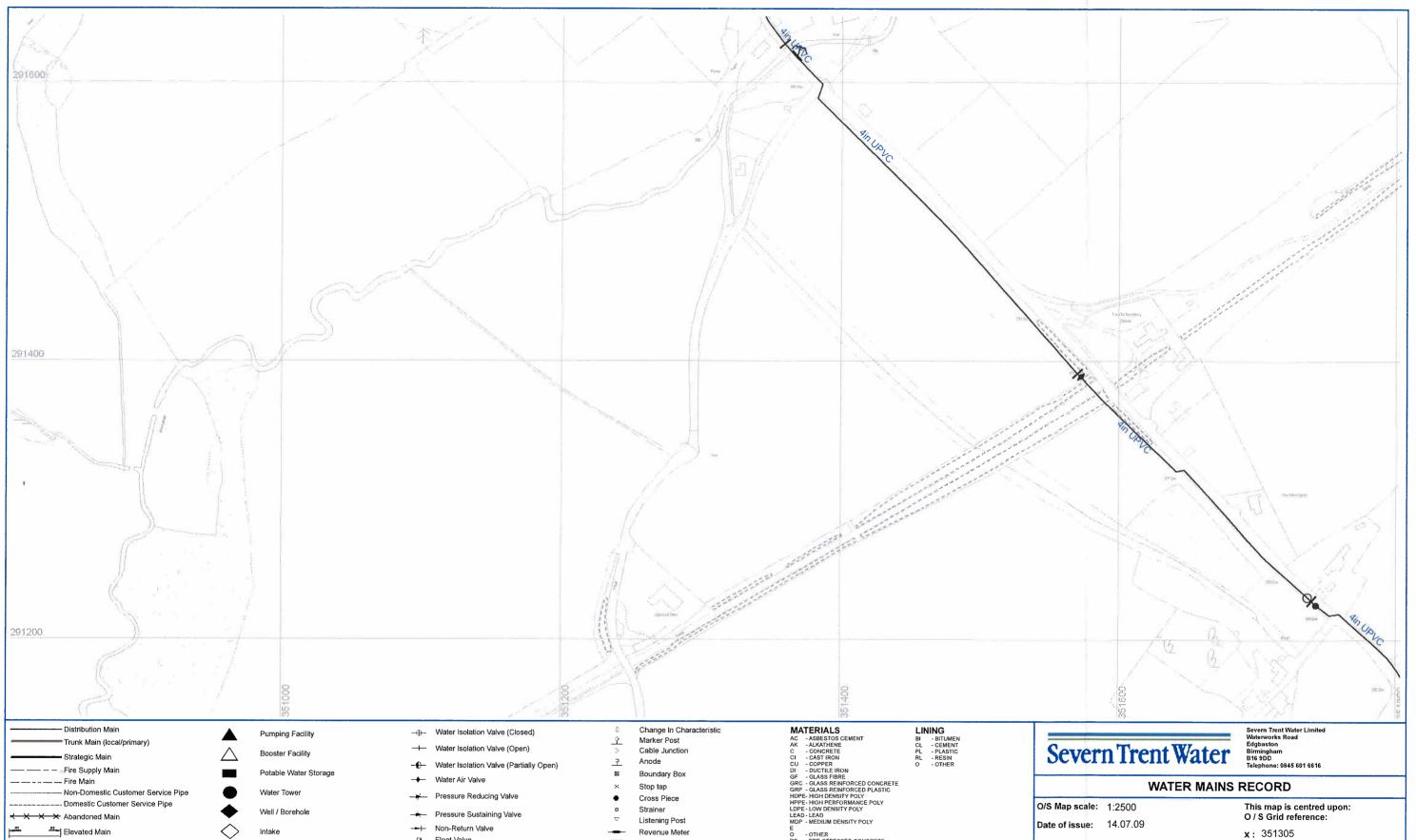
Enclosed is a copy of the plan(s) showing the approximate positions of the public water mains and/or public sewers situated within the vicinity of the land/property which is the subject of your enquiry.

The Records Management Centre can only provide plans of the location of the Company's underground assets. Therefore service pipes and drains are the responsibility of the property owner and should be anticipated during any excavation.

Please also find enclosed a copy of Severn Trent Water's General Conditions and Precautions for your information.

A VAT receipt is attached herewith together with your enquiry documentation. (Please ensure that VAT receipts are forwarded to your finance department where appropriate).

Records Management Centre



♦ ♦ Cable, Earthing ----- Cable, Optical Fibre/Instrumentation ----- Cable, Low Voltage ----- Cable, High Voltage

= Duct

Water Treatment Works / Chamber Draw-off Tower Bowser Point \boxtimes Water Facility Connection Quality Sample Point

Hydrant (Single/Double) Washout (Single/Double) Bulk Meter Water Hatch Box Pressure Tapping Insertion Flow Meter Point Water Chemical Injection Point Motive Water Point

 \square Housing, Building [K] Housing, Kiosk Housing, Other Pipe Support Structure Open Pipe Discharge End Cap SSSI Area Access Right Pre-1937 Properties

O - OTHER
O - OTHER
PC - PRE-STRESSED CONCRETE
PF - PITCH FIBRE
PSC - PLASTIC STEEL COMPOSITE
PVC - POLY VINYL CHLORIDE
RPM - REINFORCED PLASTIC MATRIX
SI - SPEUN IRON
SST - STAINLESS STEEL
UPVC- UNPLASTICISED PVC

y: 291395

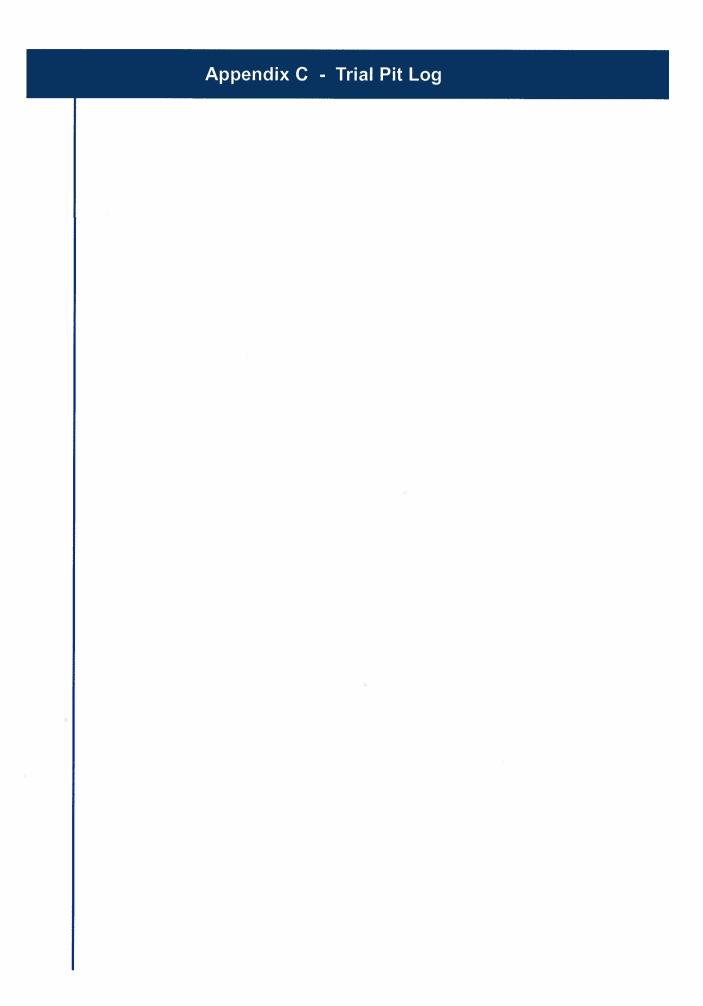
Do not scale off drawing:

- This plan and any information supplied with it is furnished as a general guide, is only valid at the date of issue and no warranty as to its correctness is given or implied. In particular this plan and any information shown on it must not be relied upon in the event of any development or works (including but not limited to executations) in the virty of SEVERN TRENT WATER assets or for the purposes of determining the suitability of a point of connection to the sewerage or
- Reproduction by permission of Ordnance Survey on behalf of HMSO. © Crown Copyright and database right 2004. All rights reserved.

Ordnance Survey licence number 100018202.

Document users other than SEVERN TRENT WATER business users are advised that this document is provided for reference purpose only and no further copies should be made from it.







STRUCTURAL_SOILS_GINT_LIBRARY.GLB!TRIAL PIT LOG - STANDARD | 761191 - BE4 BD21 BRIDGES.GPJ - v8_02 | 01/09/09 - 10:51.
Structural Soils Ltd, Branch Office - Castleford: The Potteries, Pottery Street, Castleford, West Yorkshire, WF10 1NJ. Tel: 01977-552255, Fax: 01977-552299, Web: www.soils.co.uk, Emait:north@soils.co.uk.

STRUCTURAL SOILS

TRIAL PIT LOG

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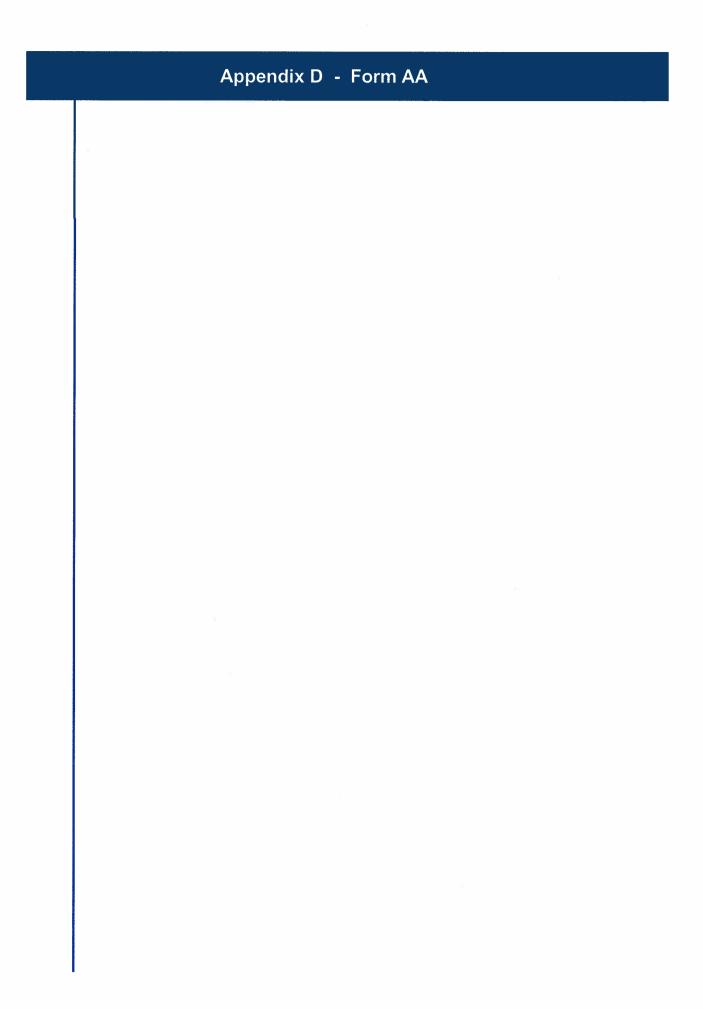
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Structural Soils Ltd, Branch Office - Castleford: The Potteries, Potteries, Pottery Street, Castleford, West Yorkshire, WF10 1NJ. Tel: 01977-552255, Fax: 01977-552299, Web: www.soils.co.uk, Email:north@soils.co.uk.

STRUCTURAL SOILS

TRIAL PIT LOG

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Group Standard

FORM 'AA' (BRIDGES)

GC/TP0356

Appendix: 4 Issue: 1

ELR/ Bridge No BUI/26

Revision: B (Nov 2000)

APPROVAL IN PRINCIPLE FOR ASSESSMENT

Bridge/Line Name:

Red Barn, Rushbury / Buildwas to Marsh

ELR/Bridge No.

BUI/26

Brief Description of Existing Bridge:

(a) Span Arrangement

The structure is a square, single span overbridge. The clear span is 7.58m (24' - 10").

(b) Superstructure Type

The bridge comprises of three internal longitudinally spanning cast iron girders, spaced at about 1.358m (4' -51/2") centres and two cast iron edge girders. Pre-cast concrete arches span between the bottom flanges of the girders and these are covered in concrete to form the bridge deck. The internal girders have an overall depth of 21" (533mm) at mid span and 16" (406mm) at supports. The edge girders have an overall depth of 20" (508mm) throughout.

(c) Substructure Type

The abutments and wingwalls are masonry gravity type structures constructed from random size stones irregularly coursed.

(d) Planned highway works/modifications at this site

None

(e) Road designation class and whether classed as a heavy load route

The bridge carries a public bridleway also used for field access. The bridleway is unpaved. The bridge deck itself is concrete with no additional surfacing. The heaviest vehicles ever likely to use the bridge are farm tractors and trailers. HGV use is low (L) and road surface condition is considered to be poor (p).

(f) Any other requirements

None.

Group Standard

FORM 'AA' (BRIDGES)

GC/TP0356

ELR/ Bridge No BUI/26

Appendix: 4 Issue: 1

Revision: B (Nov 2000)

APPROVAL IN PRINCIPLE FOR ASSESSMENT

Assessment Criteria

(a) Loadings and Speed

Dimensions and condition obtained from site measurements and reference to historic data. (See Jacobs report "VAR9-2602 Assessment Programme – BD21Assessment and Inspection Report – Bridge Ref.. BUI/26"). Assessment live loading obtained from and applied in accordance with BD21/01, assuming low HGV flow and poor road condition. The bridge is assessed for up to 40/44 tonne live loading, with reduced loading being determined where this capacity is not reached.

(b) Codes to be used

BD21/01 - "The Assessment of Highway Bridges and Structures"

(c) Proposed Method of Structural Analysis

BD21 vehicle loading will be applied to the internal girders using the simple distribution methods outlined in Chapter 2 of BA 16/97. As the concrete surface occupies the entire width of the bridge the carriageway loading will also be applied to the edge girders using the same method.

The embedment of the internal cast iron girders in concrete will permit an increase in the section modulus for live load by D/d in accordance with BD21/01 Clauses 7.12 to 7.15

The assessment will be based on the current condition of the structure as determined by the inspection. Specific allowance for any recorded section losses will be made to the appropriate component of the relevant elements.

Determination of the adequacy of the jack arches will be based upon the empirical method described in Bridgeguard 3 Current Information Sheet No 22 (Pro-forma for the empirical assessment of brick, masonry and concrete jack arches and associated ties.)

The substructure will be assessed qualitatively.

FORM 'AA' (BRIDGES)

GC/TP0356

ELR/ Bridge No BUI/26

Appendix: 4 Issue: 1

APPROVAL IN PRINCIPLE F	FOR AS	Revision: B (Nov 2000) SESSMENT
Senior Civil Engineer's Co		
None		
		,
Proposed Category for Inde	pendent	Check
Superstructure		
Substructure		
Name of Checker suggeste	d if Cat 2	2 or 3
Category 1		
The above assessment, with amer	ndmer	
,	Signe	
	Title Date	26/11/2009
Category 2 and 3	Date	
The above assessment, with ame	ndments	shown, is approved in principle:
	Signed	
	Title	
	Date	
	Signed	
	Title	
	Date	·/



Appendix E - Form BA

Group Standard

FORM 'BA' (BRIDGES)

GC/TP0356

Appendix: 4

ELR/ Bridge No BUI/26

Issue: 1 Revision: A (Dec 2005)

CERTIFICATION FOR ASSESSMENT CHECK

Assessment Group:

Jacobs Engineering UK Ltd

Bridge/Line Name:

Red Barn Overbridge / Buildwas to Marsh

Category of Check:

ELR/ Bridge No:

BUI/26

We certify that reasonable professional skill and care have been used in the assessment of the above structure with a view to securing that:

- (1) It has been assessed in accordance with the Approval in Principle as recorded on Form AA approved on 26 November 2009.
- (2) It has been checked for compliance with the following principal British Standards, Codes of Practice, BRB (Residuary) Limited technical notes and Assessment standards:

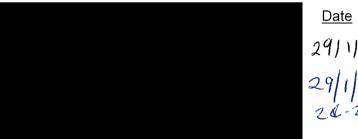
BD 21/01 - "The Assessment of Highway Bridges and Structures"

BD 16/97 - "The Assessment of Highway Bridges and Structures"

List any departures from the above and additional methods or criteria adopted, with reference and justification for their acceptance.

None

Category 1



29/1/10

20-2-9

Assessor

Assessment Checker

Authorised signatory of the firm of Consulting Engineers to whom Assessor/Checker is responsible.

Group Standard

FORM 'BA' (BRIDGES)

GC/TP0356

ELR/ Bridge No BUI/26

Appendix: 4

Issue: 1

Revision: A (Dec 2005)

CERTIFICATION FOR ASSESSMENT CHECK

Category 2 and 3 (Note: Category 1 check must also be signed)

(a) Assessment

<u>Name</u> Signature **Date** Assessor Assessment Checker Authorised signatory of the firm of Consulting Engineers to whom Assessor/Checker is responsible. (b) Check <u>Name</u> Signature **Date** Assessor Assessment Checker Authorised signatory of the firm of consulting engineers to whom Assessor/Checker is onsible.

This Certificate is accepted

Group Standard

FORM 'BAA' (BRIDGES)

GC/TP0356

Appendix: 4

ELR/ Bridge No BUI/26

Issue: 1

Revision: A (Dec 2005)

CERTIFICATION FOR ASSESSMENT CHECK

Notification of Assessment Check

Assessment Group

Jacobs Engineering UK Ltd

Bridge Name/Road No.

Red Barn Overbridge / bridleway

Line Name

Buildwas to Marsh

ELR Code/Structure No.

BUI/26

The above bridge has been assessed and checked in accordance with Standards which are listed on the appended Form BA. A summary of the results of the assessment in terms of capacity and restrictions is as follows:-

STATEMENT OF CAPACITY

Edge girders:

7.5 tonnes ALL, Group 1 fire engines

Internal girders

18 tonnes ALL

Abutments (qualitative assessment)

>18 tonnes ALL

Jack arches and west side tie bars

Full AW loading

East side tie bars

No live load capacity

Recommended Loading Restrictions

7.5 tonnes ALL and Group 1 fire engines subject to the tie bars in the outer arch east being replaced.

Description of Structural Deficiencies and Recommended Strengthening

Capacity of the bridge is limited by the edge girders. The assessment assumed that carriageway loading can be applied over the width of the bridge because it is a continuous concrete slab with no kerbs and the verges are only a manifestation of vegetation encroachment rather than a stop point for carriageway surfacing. In practice, the few vehicles that use the bridge track towards the centre; therefore 18 tonnes capacity may be achieved.

The tie bars in the east bay need to be replaced.

Some repointing work is recommended on the abutment quoins. Vegetation on the abutments and wingwalls, including a sapling establishing on the south abutment, needs to be cleared

Group Standard

FORM 'BAA' (BRIDGES)

GC/TP0356

ELR/ Bridge No BUI/26

Appendix: 4 Issue: 1

Revision: A (Dec 2005)

CERTIFICATION FOR ASSESSMENT CHECK



<u>Date</u>

29/1/10 Assessor

29/1/10

Assessment Checker

24.2.0

Authorised signatory of the firm of Consulting Engineers to whom Assessor/Checker is

responsible.

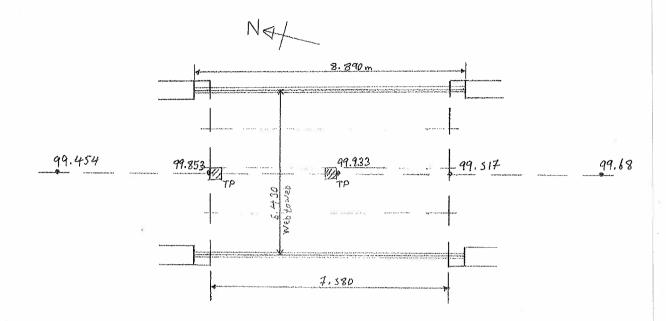
This Certificate is accepted



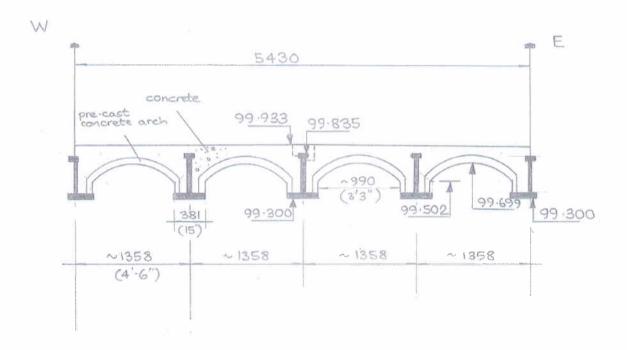
(25/2/2010)



Appendix F - Calculations



Plan



Section

CALCULATION COVER SHEET

Jacobs Reading

Project Title:	BRB (Residuary) Ltd	d - Major W	orks 2009/2012	Calc. No.:	180
Job No: B12360	АH			File:	R16
Project Manager		Subject:	BUI/26		
Designer			Red Barn Overbridge,	Rushbury,	Shropshire
Project Group	31400		BD21 Assessments		
		77			

			Date	Checked	Date	Reviewed	Date	
	Sheets	by		by		by		
Original			Jan-10		Jan-10			
Rev								
Rev		14.15 5						
Rev								
Rev					4.4			
Rev								
i (CV								

Superseded by Calculation No. Date

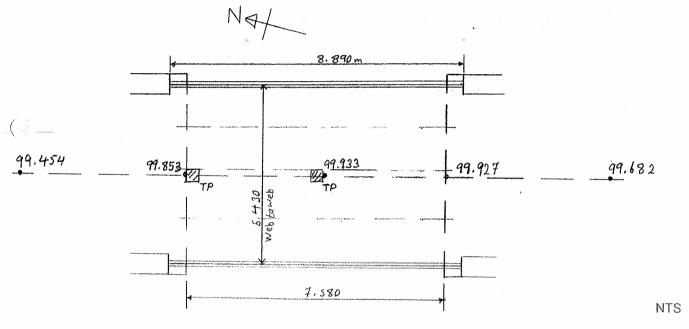
For design criteria, refer to Approval in Principle (Form AA) document

JACOBS

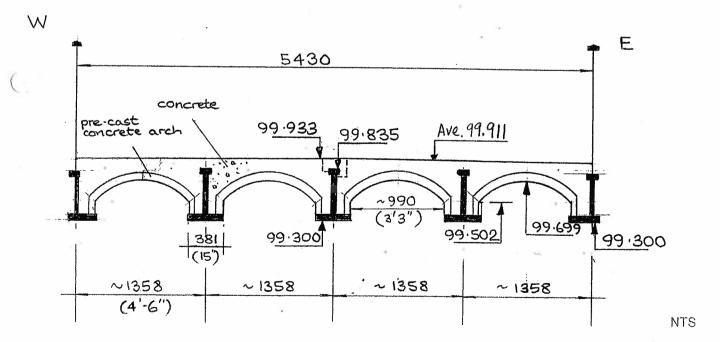
Project Title: BRB(R) Ltd - Maj	Sheet N	o: 1		
Subject: BUI/26	Calc No.	180		
Job No: B12360AH			File:	R16
Made E	Date: 01/10	Revised By:		Date:
Checked By:	Date: 1 10	Checked By:		Date:

BD21 assessment of BUI/26

Site survey by Jacobs - August 2009:

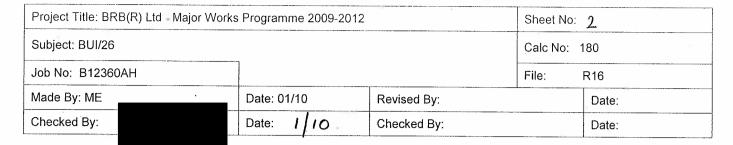


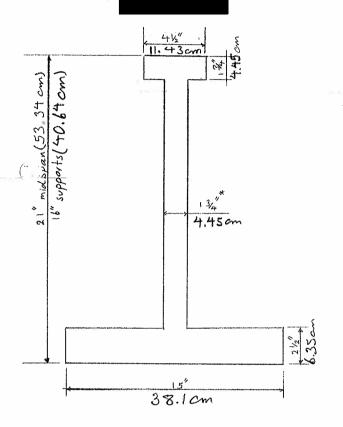
Bridge plan

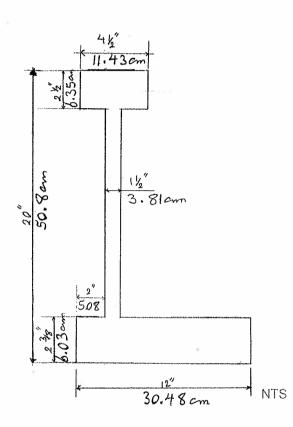


Deck cross-section









Internal girders

Edge girders



Project Title: BRB(R) Ltd - Ma	Sheet No: 4		
Subject: BUI/26			Calc No: 180
Job No: B12360AH			File: R16
Made By:	Date: 01-10	Revised By:	Date:
Checked I	Date: //10	Checked By:	Date:

Girders section properties:

Internal girders (mid-span):

Element	Dim	ension	Area	y from top	Ау	A(y-y1)^2	I=bd^3/12
μ	b(cm)	d(cm)	=)				
Top flange	11.43	4.445 /	50.8	2.223	112.92	56313.61	83.65
Web	4.445	42.545 /	189.1	25.718	4863.50	18153.47	28525.69
cottom flange	38.10	6.350 /	241.9	50.165	12136.67	51924.00	812.95
NET AREA			481.9		17113.09		
GROSS AREA			481.9				
Depth to Neutral Axis y1		35.52					
					Sum	126391.07	29422.29

lxx=

155813.37

Ztop=

4387.24

Zbot=

8741.33

Internal girders (supports):

Element	Dim	ension	Area	y from top	Ау	A(y-y1)^2	I=bd^3/12
	b(cm)	d(cm)					
Web Bottom flange	11.43 4.445 38.10	4.445 29.845 6.350	50.8 132.7 241.9	2.223 19.368 37.465	112.92 2569.31 9064.09	32751.82 9017.79 23486.16	83.65 9847.03 812.95
NET AREA GROSS AREA Depth to Neutral Axis y1		27.61	425.4 425.4		11746.32		
	<u> </u>				Sum	65255.78	10743.63

lxx=

75999.41

Ztop=

2752.38

Zbot=

5833.66



Project Title: BRB(R) Ltd - Major Works Programme 2009-2012				
		Calc No:	180	
		File:	R16	
10	Revised By:		Date:	
/10	Checked By:		Date:	
i		10 Revised By:	Calc No: File:	

Edge Gir

Element	Dime	ension	Area	y from top	Ay	A(y-y1)^2	I=bd^3/12
	b(cm)	d(cm)					
Top flange	11.430	6.350	72.6	3.175	230.44	58934.43	243.89
Web	3.810	38.418	146.4	25.559	3741.05	5467.25	18002.43
Bottom flange	30.480	6.033 /	183.9	47.784	8786.03	47740.25	557.60
NET AREA			402.8		12757.52		
GROSS AREA			402.8				
Depth to Neutral Axis y1		31.67					
		,			Sum	112141.93	18803.92

lxx=

130945.85

Ztop=

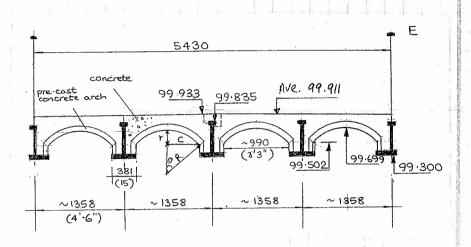
4134.65

Zbot=

6845.19

Project Title. BRB(R) Ltd Maj	Sheet No	: 6		
Subject: BUI / 26	Calc No:	180		
Job No: B12360AH			File:	R16
Made By	Date: 01/10	Revised By:		Date:
Checked	Date: 1 10 .	Checked By:		Date:

Internal girders (Dead loads)



$$R = \frac{c^2 + r^2}{2r} = 720 \text{ mm}$$

Fill area above arch springing = 1.358 x (99.911-99.502) 133789 x 10 = 0.422 m2

P. 1

P.4

Project Title: BRB(R) Ltd - Ma	Sheet No:	7		
Subject: BUI / 26	Calc No:	180		
Job No: B12360AH			File:	R16
Made By:	Date: 01/10	Revised By:		Date:
Checked	Date: 10	Checked By:		Date:

BD 21101 Table 4.1

CL. 6.5

Wt. of fill and arch borrel &

= 2300 x 0.475=1093 kg/m

wt of girder = 7200 x 0.0499 = 359 kg/m

Total factored dead load = 11.0 × 1093 + 1.0 × 359

= 1452 kg = 14,24 kN.m

Effective span:

clear span = 7.580

:: Effective span = 7.580 m + 2 x 1 x 16x25.4)

=7.715 m (Internal girders)

Effective span = 7.580 + 2x($\frac{1}{3}$ x $\frac{1}{2}$ x $\frac{20^{\circ}$ x 25.4) = 7.749 m (Edge girder)

Project Title: BRB(R) Ltd - Major Works Programme 2009-2012						8
Subject: BUI / 26				Calc No:	180	
Job No: B12360AH				File:	R16	
Made By:		Date: 01/1	9	Revised By		Date:
Checked I		Date: 1	10.	Checked By:		Date:

Dead load effects (Internal girders)

p. 7

Dead load bending moment at mid-span =
$$\omega L^2 = 14.24 \times 7.715_{8}^2 = 105.9 \text{ KN.m}$$

Dead load shear at supports = 64.9 km.

Dead load stress (bending):

P.4

$$\delta = \frac{M}{I_b} = \frac{105.9 \times 10^6}{8741.33 \times 10^3} = 12.11 \, \text{M/min}^2 \, \text{(tension)}$$

$$\delta_{c} = \frac{u}{Z_{t}} - \frac{105.9 \times 10^{6}}{4387.24 \times 10^{3}} = 24.14 \, \text{Mm}^{2} \, \text{(compression)}$$

Dead load stress (shear):

1. 2

Project Title: BRB(R) Ltd - Major	Sheet No	9		
Subject: BUI / 26	Calc No:	180		
Job No: B12360AH			File:	R16
Made By	ate: 01/10	Revised By:		Date:
Checked	ate: [[(0	Checked By:		Date:

Live load Linternal girders):

P. 1

Parapet spacing = 5.430

clear carriageway < 5.0m (restricted by width between stone pilasters)

BD 21/01 Table 5.1 .. No of notional lanes = 1

girders spacing= 1.358m, Effective span=7.715m

BA16/97 Table 2/2(a)

Use BA16/97 distribution chart for internal girders

K1= 0,49

BA21/01

HA load = 336(1)0.67 = 85.5 KN/m

MA KEL = 120 KN

5.23

Adjustment factor = 3.65/2.5 = 1.46

Table 3.1

P_= 1.0

HA adjusted live load moment $= \left[\frac{85.5 \times 7.715^{2}}{2} + \frac{120 \times 7.715}{4} \right] \times \frac{0.49}{1.41} \times 1.0 = 291.2 \text{ kM.m}$

HA adjusted live load shear:

13A16197 Cl. 2.8 51= K SU +0.5 SK

 $S_L = \left(\frac{85.5 \times 7.715}{2 \times 1.46}\right) \times 0.49 + 0.5 \times \left(\frac{120}{1.46}\right) = 151.8 \text{ kN}$

Project Title: BRB(R) Ltd - Ma	Sheet N	lo: IQ		
Subject: BUI / 26	Calc No	o: 180		
Job No: B12360AH			File:	R16
Made By:	Date: 01/10	Revised By:	\\	Date:
Checked I	Date: 1/10	Checked By:		Date:

BO 21101	Reduction factor "k" for "low" HGV use and "poor"
	road condition (fig 5.4)
t kij falle. Salijak prak	K= 0.87 (40 tonnes)
	: live road effects:
	M = 291.2 x 0.87 = 253.3 kmm
	V = 151.8 x 0.87 = 132.1 km
130 21/01 cl. 7.13	Section modulus may be increased for line load stress by
	factor of Dy
	$d = 21'' \times 25.4 = 533 \text{mm} / (at mid span)$
P.6	D = (99.933-99.300) x 1000= 633 mm
cl. 7.14	Of = 1.188 < 2.0
	Live 10ad stress (bending -402)
	$\delta_t = \frac{\mathcal{U}}{Z_b} = \frac{253.3 \times 10^b}{1.188 \times 8741.33 \times 10^3} = 24.44 \text{ M/mm}^2 \text{ (tension)}$
	$\delta_{C} = \frac{M}{I_{t}} = \frac{253.3 \times 10^{6}}{1.188 \times 4387.24 \times 10^{3}} = 48.6 \frac{M}{\text{mm}^{2}}$ (compression)

P. 8
$$\delta = \frac{132.1 \times 10^{2}}{18065} = 7.312 \frac{1}{mm^{2}}$$

Project Title: BRB(R) Ltd - Maj	Sheet No	: 11		
Subject: BUI / 26				180
Job No: B12360AH			File:	R16
Made By	Date: 01/10	Revised By:		Date:
Checked	Date: 1 10 ·	Checked By:		Date:

Man permissible live lead tensile stress P_ = 24.6-0.44 Pd = 24.6-0.44 x 12.11 = 19.27 N/min2 BO 21/01 4.10 , P.8 fc = 19.6 -0.76 fd = 19.6 -0.76 x 12 11 = 10.39 Mmm2 => P_= 19.27 1/mm2 > 24.4 1/mm2 .. Internal girders are not adequate for 40 tonnes loading * Assessed live load vesistance (tension) 19.27 x (8741.33 a1.188) x 103 = 200.1 KNm For 26 tonnes loading: k=0.85 = M= 291,2x0.85=247.5 \ 200.1 Fig 54 6 P.9 : Internal girders are not adequate for 26 tomes loading. Fig 5.4 18 tonnes loading k= 0.67 $M = 291.2 \times 0.67 = 195.1 \text{ kmm} < 200.1 \text{ kmm} : 0 \text{ k}$ Fig 5.4 Group I FE loading: k=0.49 :. M = 2911.2 x 0.49 = 142.7 km < 188.4 kmm., 0k .: Internal girders are adequate for 18th loading in tensile bending.

Project Title: BRB(R) Ltd - Ma	Sheet No: 2		
Subject: BUI / 26			Calc No: 180
Job No: B12360AH			File: R16
Made By:	Date: 01/10	Revised By:	Date:
Checked	Date: 1/10	Checked By:	Date:

Permissible stress(compression)

P 8

 $\begin{cases} f_{L} = -43.9 + 0.79 & fd = -43.9 + 0.79(-24.14) = -62.97 \frac{M_{mm}^{2}}{M_{mm}^{2}} \\ f_{L} = -81.3 + 3.15 & fd = -81.3 + 3.19(-24.14) = -158.3 \frac{M_{mm}^{2}}{M_{mm}^{2}} \\ fd + f_{L} < 154 \frac{M_{mm}^{2}}{M_{mm}^{2}} \end{cases}$

P 10

P 11,12

.. Permissible live load stress = 154-24.97= 129.0 1/mm² > 48.6 "okin40 tcomp."

.. Internal girders are adequate for 18 tonnes loading in bending.

shear check:

B021/01 4.11 6P8

Permissible shear stress= $24.6-0.449d = 24.6-0.44\times3.04$ = $23.3 \times 10 > 7.312 \text{ Mm/2} OK$

P.10

7.312+3.04=10.35 < 42 Mmm 2

:. Internal girders are adequate for 40 tonnes 10 ading in shear

P.8

* Assessed live load shear = 23.3 x 18065/1000 = 420.9 km

Project Title: BRB(R) Ltd - Major Wor	2	Sheet No:	13	
Subject: BUI / 26		Calc No:	180	
Job No: B12360AH		File:	R16	
Made By:	Date: 01/10	Revised By:		Date:
Checked	Date: //10	Checked By:		Date:

Edge girder (dead 10ad)

wt of corrugated steelparapel (Imm Assumed thickness)
= 7850 kg/m3 x 1.34 m x 0.007 x 1.1 = 81 kg/m
LApen, connections

P.5 Wt. of edge girder = 7200 x (402.8 x 10 1) x 1.1 = 319 kg/m

Wt. of fill = 1093/2 = 547 kg/m

Total factored dead load = 81+319+547=947 kg/m

= 9.29 KN/m

Project Title: BRB(R) Ltd - Major W	Sheet No: 14		
Subject: BUI / 26	Calc No: 180		
Job No: B <u>12360AH</u>			File: R16
Made By:	Date: 01/10	Revised By:	Date:
Checked E	Date: 1 10	Checked By:	Date:

Dead load effects (Edge girders)

Dead load bending moment at mid-span

P. 13,7 =
$$\frac{\omega L^2}{8} = 9.29 \times 7.749_{/8}^2 = 69.73 \text{ k.v.m}$$

Dead load shear at supports = WL, = 9.29x 7.749/2=360 KN

Dead load stress (bending):

P 5

$$\sigma_{t} = \frac{M}{Z_{b}} = \frac{69.73 \times 10^{6}}{6845.19 \times 10^{3}} = 10.19 \, \text{M/mm}^{2} \, \text{(tension)}$$

$$O_c = \frac{M}{I_t} = \frac{69.73 \times 10^6}{4134.65 \times 10^3} = 16.86 \text{ N/mm}^2 \text{ (compression)}$$

Dead load stress (shear):

$$\sigma = \frac{36.0 \times 10^3}{19355} = 1.86 \text{ M/mm}^2$$

 Project Title: BRB(R) Ltd - Major Works Programme 2009-2012
 Sheet No: 15

 Subject: BUI / 26
 Calc No: 180

 Job No: B12360AH
 File: R16

 Made By
 Date: 01/10
 Revised By: Date:

 Checked
 Date: 1 10
 Checked By: Date:

Live load L Edge girder)

L= 7.75 m

BA 21101 CL 5.18 , P.7 HA load = 336 (1)0.67 = 85.22 km/m/2.5mlane
HA KEL = 120 KN / 2.5mlane

5. 23

AF = 3.65/2.5 = 1.46 8P_=1.0

P. 1

Girder spacing = 1.358m

BA16197 Table 213 (a) Lateral distribution factor = 0.415

HA adjusted live load moment $= \left[\frac{85.22 \times 7.749^{2}}{8} \right] \times \frac{0.415}{1.44} \times 1.0 = \frac{247.9 \text{ kW.m}}{1.44}$

HA adjusted live load shear = $\left[\frac{85.22 \times 7.748}{2} + 120\right] \times \frac{0.415}{1.44} \times 1.0 = 128.0 \text{ kN}$

Project Title: BRB(R) Ltd - Ma	Sheet No	o: 16		
Subject: BUI / 26			Calc No:	180
Job No: B12360AH			File:	R16
Made By	Date: 01/10	Revised By:		Date:
Checked	Date: 1 10	Checked By:		Date:

made by		23.3.0171	<u> </u>	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	
Checked		Date: 1	10	Checked By:	Date:
P.10	k = 0.87 Live 10a				
	м= 247.9			- Kw.m	
	V = 128.1	0.87	= 111.4	kW.	
	Live load:	stress in	, bending	(40 tonne)	
	~			31.51 Mm.2 (Lension)	
	6c = 1 = 3	4134.65	$\frac{1}{5\times10^3}=\frac{5}{5}$	52.17 1/mm (compression)	
	live load	stremin	shearl	40 tonne)	
P.14	$\beta = \frac{111.4 \times 10^{-10}}{19.355}$	$\frac{0^3}{2} = 5.7$	6 M/mm 2		
	Man permis			A STATE OF THE STA	PRODUCTION CONTRACTOR
P.14	L= 24.6 -	0.44 fo	1 = 24.6	- 0.44x10.19 = 20,12 /mm2	\$ 31.51 M
	Edge g	rirders d	are nat	adequate for 40thoading	7
	* Assesse	ed live	load re	sistance = 20.12x6845.10	1/03 = 137.7 kw.m
Table 5.4	k = 0.6				The Control of the Co
P.15	u=247.9	x 0.67	= 166.	1 kN.m & 137.7 kN.m	Fail for 18th loading
Table 5.4	K=0.49	L Gr	oap 1		
Table 5.4	M=247.9 k=0.38	7 x 0.40	1=121.5 19)⇒ U	5 kum < 137.7 kum <u>(</u> =247.9 x0.38 = <u>94.2</u> <137.7	ok for Group 1 FE k.N.m Ok for 7.5t load



Project Title: BRB(R) Ltd - Majo	Sheet No: 17		
Subject: BUI / 26	Calc No. 180		
Job No: B12360AH			File: R16
Made By:	Date: 01/10	Revised By:	Date:
Checked	Date: 1/10 .	Checked By:	Date:

	Man. Permissible live load compression streps:
BD21/01 CL. 4.10	$f_2 = -43.9 + 0.79 f_d = -43.9 + 0.79 \times (-16.86) = 57.21 Mmm2$
6 P. 14	f _L = -81.3+3.15 f ₀ l = -81.3+3.15x(-16.86) = -134.41 //m.
	$f_{2} + f_{d} < 154$ 134.1+16.86 = 150.96<154
·- 12:16	$f_2 = 134.41 > 52.17 \text{N/mm}^2$ ii Ok for 40 ^t loading
	". Edge girders are adequate for Group 1 FE" in bending (Limited by tensile stress)

shear check:

BD21/01 4.11 , P.14

Permissible shear stress = 24.6-0.44 gd = 24.6-0.44x1.86

P.16

= 23.18 * N/m > 5.76 N/mm2

2318+1.86 = 25.04 < 46 Mmm2 : OK

:. Edge girders are adequate for 40 tonnes loading in shear

P. 14

* Assessed live load shear = 23.18x 19355 x 10-3=448.6 km

PRO FORMA FOR EMPIRICAL ASSESSMENT OF BRICK, MASONRY AND CONCRETE JACK ARCHES AND ASSOCIATED TIES

(To be included with the Assessment Report Calculations)

BRIDGE NAME:	Red Barn Overbridge	
RAILTRACK NO:	BUI/26	

Assessment should include completion of all three Sections even where Section 1 has shown the bridge deck to be non-compliant.

SECTION 1 CHECKS FOR COMPLIANCE WITH 24 T CONFIGURATION REQUIREMENTS

		Compliant Yes/No
What is maximum clear span of the arch Non-compliant if greater th	0.990m an 2.0m	<u>Yes</u>
Do jack arches spring from bottom flanges of beams? The pre-cast units sit on the bottom beams, if not, non compliant though the actual springing policy.	oottom flage it is about 15	s <u>No</u> Omm highe
What is the beam spacing?	b=1.358m	
What is the rise of the arch?	r _c =0.197m	Yes
Gross aspect ratio	b/r _c =6.9	
Non-compliant if greater th	an 10	
What is the arch barrel thickness (including concrete fill above) and how is it derived ie from record drawings or site investigation? Non-compliant if thickness	<u>No</u>	

CAIC NO 180 RIE RIE ROB NO B12360 AV

PRO FORMA FOR EMPIRICAL ASSESSMENT OF BRICK, MASONRY AND CONCRETE JACK ARCHES AND ASSOCIATED TIES

(To be included with the Assessment Report Calculations)

BRIDGE NAME:	Red Barn Overbridge	
RAILTRACK NO:	BUI/26	

Assessment should include completion of all three Sections even where Section 1 has shown the bridge deck to be non-compliant.

SECTION 2 CHECKS FOR DEFICIENCY

Type No	Deficiency		
1	What is the backing material? Is it structural? All concrete, Yes Does the structural backing extend to at least the crown level of the arch extrados? If not, then fail (1) (4)	Pass	
	Height of structural fill above crown $\underline{d}_{\underline{f}} := 136 \underline{mm}$		
	What is effective shear depth of deck?	i	
	(= arch rise + barrel thickness + depth of structural fill above crown of extrados) $\underline{D}_{\underline{s}} := \underline{r}_{\underline{c}} + \underline{d} + \underline{d}_{\underline{f}}$ $\underline{D}_{\underline{s}} := 212 \underline{mm}$	Fail	
	ls <u>D</u> _s ≥"minimum requirements of Fig 1 " Fail if < Fig 1	ran	
	All concrete construction is acceptable		
	€ 500 E 500		
	Minimum Permissible Effective Shear Depth (mm) 300 200 200 200 200 200 200 200 200 200		
	Minimum Permissible fective Shear Depth (min man man man man man man man man man ma	#	
	Shea 300		
	Le Ctivinio 200 Le Ctivinio 20		
	100		
	0		
	0 500 1000 1500 2000 2500 Clear Arch Span (mm)		
	Figure 1		

	i i					
2	Do jack arches span longitudinally (eg in half through girder construction) or transversely between longitudinal girders? For longitudinal spanning jack arches, ignore following questions on ties/lateral restraint and state N/A.					
	Are ties provided in edge bays of transversely spanning jack	arches?	yes -			
	If yes, go to 3a/3b If not, fail unless edge bay	/ is 'hard' (see 5)	,00	<u>pass</u>		
3a	What is the cross sectional area of one Diameter of tie tie? (allowing for corrosion losses)	s east bay		st ba	ed ti Y	es
CI	Therefore Area	$\underline{\mathbf{A}} := 132.7 \; \underline{\mathbf{mm}}^2$	$A = A^2$) nm2		
	What is number of ties per beam length?	<u>n</u> := 3	n = 3	58	2	04
	What is the clear skew span?	<u>L</u> := 7.580 <u>m</u>	As= 2		n/m	OK
	Specific area of tie $\underline{\underline{A}}_{\underline{S}} := \frac{(\underline{n}+1) \cdot \underline{\underline{A}}}{\underline{\underline{L}}}$	$\underline{A}_{\underline{S}} := 70.0 \frac{\underline{mm}^2}{\underline{m}}$		<u>Fail</u>		
	Non-compliant if less ti	han 260mm²/m				
	What is maximum tie spacing?	$\underline{S} := 1.9 \ \underline{m}$				
	Non-compliant if greate	er than 2.5m for cas	t iron	Pass		
3b	What is the cross sectional area of one Diamensions of the tie? (allowing for corrosion losses)	the tie $\frac{dt1}{dt2} := 0 \underline{t}$ $\frac{dt2}{dt2} := 0 \underline{t}$				
	Therefore Area <u>A</u> := <u>dt1</u> - <u>dt2</u>	$\underline{\mathbf{A}} \equiv 0 \cdot \mathbf{m}^2$				
WI/ST	What is number of ties per beam length?	<u>n</u> := 0				
	What is the clear skew span?	<u>L</u> := ø <u>m</u>		<u>NA</u>		
	Specific area of tie $\underline{\underline{A}}_{\underline{S}} := \frac{(\underline{n}+1) \cdot \underline{\underline{A}}}{\underline{\underline{L}}}$	$\underline{\mathbf{A}}_{\underline{\mathbf{S}}} \equiv 0 \cdot \mathbf{m}$				
	Non-compliant if less to	han 260mm²/m				
	What is maximum tie spacing?	<u>S</u> := n <u>m</u>				
	Non-compliant if greater t	han 3.0m for wroug	ht iron/steel			
4	Are ties located within crown of external arch? No If so, then fail CI or pos	ssible fail for WI/ste	el	Pass		
	,, co, aron van or or por			1	l	

Notes: (1) Results also in loss of D/d (composite action) for cast iron beams

(4) A trial hole should be undertaken to confirm the existence of structural backing if there is any doubt. FRE BIZ360 AIM

PRO FORMA FOR EMPIRICAL ASSESSMENT OF BRICK, MASONRY AND CONCRETE JACK ARCHES AND ASSOCIATED TIES

(To be included with the Assessment Report Calculations)

BRIDGE NAME:	Red Barn Overbridge
RAILTRACK NO:	BUI/26

Assessment should include completion of all three Sections even where Section 1 has shown the bridge deck to be non-compliant.

SECTION 3 CHECKS FOR DEFICIENCY

Type No	Defect	Empirical Assessment		Pass/ Fail	
		CI Decks	WI/Steel Decks		
6	Rotation of supporting beam?	Fail	<u>NA</u>	Pass	
7	Horizontal displacement of supporting beam?	Fail	<u>NA</u>	<u>Pass</u>	
8	Inadequate support to springings eg corrosion of bottom flange of supporting beam over a significant length, missing bedding mortar? No	Possible Fail	<u>NA</u>	<u>Pass</u>	
9	Transversely bowed bottom flange of supporting beam? No	Fail	<u>NA</u>	<u>Pass</u>	
10	Cracking at crown of arch owing to spreading of springings (other than 12, 13)? No	Fail	<u>NA</u>	<u>Pass</u>	
11	Distortion and any associated cracking of jack arch barrel?	Fail	<u>NA</u>	<u>Pass</u>	
12	Arch crack resulting in substructure crack? $\underline{\text{No}}$	Fail	<u>NA</u>	<u>Pass</u>	
13	Substructure crack or other distress resulting in crack to jack arch?	Possible Fail	NA NA	Pass	

Notes:

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